GENERAL STRUCTURAL NOTES

1.0 ALL CONSTRUCTION SHALL CONFORM TO THE INDIANA BUILDING CODE 2014. REFERENCE TO OTHER STANDARD SPECIFICATIONS OR CODES SHALL MEAN THE LATEST STANDARD OR CODE ADOPTED AND PUBLISHED.

1.1 THE CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS, DIMENSIONS AND ELEVATIONS BEFORE STARTING WORK OR FABRICATION. NOTIFY THE ENGINEER OF RECORD OF ANY DISCREPANCIES. ALL WORK COMPLETED THAT DOES NOT CONFORM TO THE STRUCTURAL DRAWINGS SHALL BE CONSIDERED AT RISK.

1.2 THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE DESIGN, ADEQUACY, AND SAFETY OF ERECTION BRACING, SHORING, AND TEMPORARY SUPPORTS. THE STRUCTURAL ELEMENTS ARE NOT STABLE UNTIL THE STRUCTURE IS COMPLETE.

1.3 ALL WORK SHALL CONFORM TO APPLICABLE FEDERAL, STATE AND LOCAL LAWS.

1.4 THE CONTRACTOR SHALL LOCATE ALL UTILITIES PRIOR TO DIGGING, BORING OR EXCAVATING SOILS OR DRILLING, BORING OR CUTTING CONCRETE.

1.5 DIMENSIONS ON STRUCTURAL DRAWINGS ARE TO BE CHECKED AGAINST ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS AS WELL AS AGAINST FIELD CONDITIONS BY CONTRACTORS.

1.6 UNLESS NOTED OTHERWISE, DETAILS, SECTIONS, AND NOTES ON THE DRAWINGS ARE INTENDED TO BE TYPICAL FOR SIMILAR CONDITIONS.

1.7 THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE LOCATION AND PLACEMENT OF INSERTS, HANGERS, SLEEVES, DUCTWORK, PADS, AND ANCHOR RODS THAT ARE REQUIRED BY MECHANICAL EQUIPMENT.

1.8 IF DISCREPANCIES APPEAR ON THE CONTRACT DOCUMENTS, OR BETWEEN THE CONTRACT DOCUMENTS AND EXISTING CONDITIONS, THE CONTRACTOR SHALL REQUEST AN INTERPRETATION FROM THE ARCHITECT BEFORE BIDDING. IF THE CONTRACTOR FAILS TO MAKE SUCH REQUEST, IT IS PRESUMED THAT BOTH PROVISIONS WERE INCLUDED IN THE BID AND THE ARCHITECT SHALL DETERMINE WHICH OF THE CONFLICTING REQUIREMENTS SHALL GOVERN. THE CONTRACTOR SHALL PERFORM THE WORK AT NO ADDITIONAL COST TO THE OWNER IN ACCORDANCE WITH THE ARCHITECT'S DETERMINATION.

1.9 REVIEW OF SHOP DRAWINGS AND OTHER SUBMITTALS BY THE STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY TO REVIEW AND CHECK SHOP DRAWINGS BEFORE SUBMITTAL TO THE STRUCTURAL ENGINEER. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS.

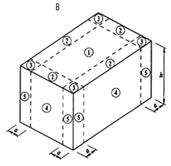
1.10 PLAN AND SPECIFICATIONS SHALL BE PROVIDED FOR REVIEW AND APPROVAL FOR PRE-ENGINEERED ROOF AND BUILDING SYSTEMS, WHICH ARE SEALED BY A LICENSED STRUCTURAL ENGINEER IN THE STATE WHERE THE PROJECT IS LOCATED PRIOR TO THE ERECTION OF THE STRUCTURES, AS DEFERRED SUBMITTAL.

STRUCTURAL DESIGN CRITERIA

LOCATION	WEST LAFAYETTE, IN
RISK CATEGORY	II
DEAD LOAD:	
DEAD LOAD INCLUDING ROOFING/INSULATION	10 PSF
COLLATERAL DEAD LOAD	10 PSF
CANOPY DEAD LOAD	5 PSF
LIVE LOAD:	
ROOF LIVE LOAD	20 PSF
CANOPY LIVE LOAD	20 PSF
SNOW LOAD:	
GROUND SNOW LOAD (P_g)	20 PSF
IMPORTANCE FACTOR (I_s)	1.0
WIND DESIGN:	
ULTIMATE WIND SPEED, 3 SECOND GUST	V = 115 MPH
IMPORTANCE FACTOR (I_w)	I = 1.0
EXPOSURE	B

COMPONENT & CLADDING DESIGN WIND PRESSURE

	ZONE	TRIBUTARY AREA (FT^2)		
		A=10	A=50	A=100
WALLS	4	+22,-24	+20,-22	+19,-20
	5	+22,-29	+20,-25	+19,-23
ROOF	1	+16,-24	+16,-23	+16,-22
	2	+22,-40	+20,-30	+19,-26
	3	+22,-40	+20,-30	+19,-26



a = 5'-9"

ALL PRESSURES ARE PROVIDED IN PSF
"+" INDICATES POSITIVE PRESSURE (TOWARD THE SURFACE) &
"-" INDICATES NEGATIVE PRESSURE (AWAY FROM THE SURFACE)

SEISMIC DESIGN:	
IMPORTANCE FACTOR (I_E)	1.0
SITE CLASS	D
SEISMIC DESIGN CATEGORY	B
S_S	0.14
S_1	0.079
S_{DS}	0.149
S_{D1}	0.126
SEISMIC FORCE RESISTING SYSTEM	LIGHT-FRAME WOOD WALLS SHEATHED WITH WOOD STRUCTURAL PANELS RATED FOR SHEAR RESISTANCE
R	6.5
Ω_0	3
C_d	4
SEISMIC RESPONSE COEFFICIENT (C_S)	0.238
FOUNDATION DESIGN:	
ALLOWABLE SOIL BEARING PRESSURE = 1,500 PSF (CONTRACTOR TO VERIFY IN FIELD)	
THE FOUNDATION DESIGN IS BASED ON PRESUMPTIVE ALLOWABLE BEARING PRESSURE AS PER BUILDING CODE AS LISTED IN 1.0. TABLE 1806.2	
DEFLECTION CRITERIA:	
LIVE LOAD DEFLECTION	L/360
TOTAL LOAD DEFLECTION	L/240

FOUNDATION & SLAB ON GRADE

2.0 A GEOTECHNICAL ENGINEER LICENSED IN THE PROJECT STATE SHALL INSPECT THE CONDITION AND ASSURE THE ADEQUACY OF ALL SUB GRADES, FILLS AND BACK FILLS BEFORE PLACEMENT OF FOUNDATIONS, FOOTINGS, SLABS, AND WALLS. KP&J ARCHITECTS AND ENGINEERS, LLC SHALL BE NOTIFIED IMMEDIATELY OF ANY NON-CONFORMING WORK.

2.1 WHEN FOOTING STEPS ARE REQUIRED, THEY SHALL BE NO STEEPER THAN ONE VERTICAL TO TWO HORIZONTAL.

2.2 PROVIDE 6 MIL MINIMUM POLYETHYLENE VAPOR BARRIER BENEATH FLOOR SLAB. LOCATE CONSTRUCTION JOINTS AT CONTROL JOINTS TO MAXIMUM EXTENT POSSIBLE.

2.3 BACKFILL SHALL NOT BE PLACED AGAINST RETAINING WALLS UNTIL THE WALLS HAVE ACHIEVED THEIR DESIGN STRENGTH AND THEIR LATERAL SUPPORT ELEMENTS ARE INSTALLED.

2.4 CONTROL JOINTS SHALL BE SAWCUT AFTER CONCRETE SURFACE HAS HARDENED ENOUGH SO THAT IT WILL NOT BE EASILY MARRED OR RAVELED. THE IDEAL TIME TO SAWCUT TYPICALLY OCCURS WITHIN 4 TO 12 HOURS AFTER CONCRETE FINISHING HAS BEEN COMPLETED. SAWCUT TIMING DEPENDS ON NUMEROUS FACTORS SUCH AS WEATHER CONDITION AND IS BEST DETERMINED ONSITE. TRIAL SAWCUTS MAYBE PERFORMED TO DETERMINE IF CONCRETE IS READY. SAWCUTTING TOO EARLY CAUSES RAVELING, OR THE DISLODGING OF AGGREGATE, WHICH RESULTS IN JOINT SPALLS. SAWCUTTING TOO LATE RESULTS IN UNCONTROLLED CRACKING. BEGIN SAWCUTTING AS SOON AS RAVELING STOPS DURING TRIAL CUTS.

FOUNDATION & SLAB ON GRADE (CONT.)

2.5 ALL SOIL SUPPORTED FOOTINGS SHALL BE FOUNDED UPON UNDISTURBED, NATURAL SOIL SUBGRADE OR ON THOROUGHLY TESTED AND APPROVED FILL BY GEOTECHNICAL ENGINEER. THE FOOTING ELEVATIONS AND SOIL BEARING CAPACITIES AS SHOWN ON THE DRAWINGS ARE ESTIMATED. FINAL, EXACT ELEVATIONS AND SOIL BEARING CAPACITIES SHALL BE FIELD DETERMINED AND VERIFIED BY THE OWNER'S SOIL TESTING LABORATORY AND REVIEWED BY THE ARCHITECT/ENGINEER DURING CONSTRUCTION.

2.6 THE SOIL SUBGRADE FOR ALL FOOTINGS AND SLABS SHALL BE INSPECTED AND APPROVED BY THE OWNER'S TESTING LABORATORY IMMEDIATELY PRIOR TO PLACING CONCRETE.

2.7 ALL FOOTING AND SLAB SUBGRADES, INCLUDING PIT SLABS, SHALL BE COMPACTED TO 95 PERCENT OF STANDARD PROCTOR (ASTM D698) MAXIMUM DENSITY AT OPTIMUM MOISTURE CONTENT, AS REQUIRED.

2.8 ALL ORGANIC AND/OR OTHER UNSUITABLE MATERIALS SHALL BE REMOVED FROM SUBGRADE AND BACK FILL AREAS AND BACKFILLED WITH SELECT FILL, COMPACTED TO 98 PERCENT OF STANDARD PROCTOR (ASTM D698) MAXIMUM DENSITY AT OPTIMUM MOISTURE CONTENT.

2.9 DO NOT UNDERMINE EXISTING CONSTRUCTION.

2.10 PLACE BACKFILL SIMULTANEOUSLY ON BOTH SIDES OF FOUNDATION WALLS.

2.11 NO MUD SLABS, FOOTINGS, OR SLABS SHALL BE PLACED ONTO OR AGAINST SUBGRADE CONTAINING FREE WATER, FROST, OR ICE.

2.12 THE CONTRACTOR SHALL PROVIDE ALL NECESSARY MEASURES TO PREVENT ANY FROST OR ICE FROM PENETRATING ANY FOOTING OR SLAB SUBGRADE BEFORE AND AFTER PLACING OF CONCRETE UNTIL SUCH SUBGRADES ARE FULLY PROTECTED BY THE PERMANENT BUILDING STRUCTURE. SUCH PROTECTION IS ADDITIONAL SCOPE.

2.13 THE CONCRETE FOR EACH ISOLATED FOOTING SHALL BE PLACED IN ONE (1) CONTINUOUS PLACEMENT.

2.14 ALL PERIMETER WALL AND COLUMN FOOTINGS SHALL BEAR A MINIMUM OF 3'-0" BELOW GRADE.

2.15 TEST PITS ARE REQUIRED FOR SOIL CLASSIFICATION AND TO LOCATE GROUND-WATER TABLE, AND VALUES USED AS THE BASIS FOR DESIGN DO NOT EXCEED THE PRESUMPTIVE VALUES IN TABLE 1806.2 (1) OF BUILDING CODE AS LISTED IN 1.0.

2.16 WHERE QUESTIONABLE SOIL CONDITIONS ARE ENCOUNTERED, THE BUILDING OFFICIAL SHALL REQUIRE THAT THE NECESSARY INVESTIGATIONS BE MADE BY A GEOTECHNICAL ENGINEER IN ACCORDANCE WITH THE BUILDING CODE. A REPORT SHALL BE PROVIDED TO THE BUILDING OFFICIAL AS DESCRIBED IN BUILDING CODE AS LISTED IN 1.0.

REINFORCED CONCRETE

3.0 ALL WORK SHALL CONFORM TO ACI 301, "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS". DESIGN IS BASED ON ACI 318 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE.

3.1 UNLESS NOTED OTHERWISE, ALL CONCRETE SHALL HAVE THE FOLLOWING MINIMUM 28 DAY COMPRESSIVE STRENGTHS:

FOUNDATIONS & WALLS EXPOSED TO FREEZE THAW CYCLE	4500 PSI (NORMAL WEIGHT)
FOUNDATIONS & WALLS NOT EXPOSED TO FREEZE THAW CYCLE	4000 PSI (NORMAL WEIGHT)
SLAB-ON-GRADE	4000 PSI (NORMAL WEIGHT)
ELEVATED SLABS	4000 PSI (LIGHTWEIGHT- 115 LB/CF MAXIMUM DENSITY)

3.2 THE PROPOSED MATERIALS AND MIX DESIGN SHALL BE FULLY DOCUMENTED AND BE PROVIDED TO KP&J ARCHITECTS & ENGINEERS, LLC OR OWNER UPON REQUEST. THE CONTRACTOR HAS THE SOLE RESPONSIBILITY OF OBTAINING THE REQUIRED DESIGN STRENGTH.

3.3 USE OF CALCIUM CHLORIDE, CHLORIDE IONS, OR OTHER SALTS IN CONCRETE IS NOT PERMITTED.

3.4 UNLESS NOTED OTHERWISE, SAMPLES FOR STRENGTH TESTS OF EACH CLASS OF CONCRETE PLACED EACH DAY SHALL BE TAKEN BY THE TESTING AGENCY NOT LESS THAN ONCE A DAY, NOR LESS THAN ONCE FOR EACH 100 CUBIC YARDS OF CONCRETE, NOR LESS THAN ONCE FOR EACH 5000 SQUARE FEET OF SURFACE AREA FOR SLABS AND FOUNDATIONS. SAMPLE CONCRETE IN ACCORDANCE WITH ASTM C172. PERFORM THE FOLLOWING TESTS IN ACCORDANCE WITH THE INDICATED STANDARD.

SUMP:	ASTM C143
AIR CONTENT:	ASTM C173
COMPRESSIVE STRENGTH:	ASTM C39 (ONE CYLINDER AT 7 DAYS, 2 CYLINDERS AT 28 DAYS, AND ONE SPECIMEN HELD IN RESERVE)

3.5 REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60 UNLESS NOTED OTHERWISE.

3.6 WELDED WIRE FABRIC (MESH) SHALL CONFORM TO ASTM A185 AND SHALL BE PROVIDED IN FLAT SHEETS (ROLLS ARE NOT PERMITTED). LAP TWO SQUARES AT SPLICES.

3.7 TIE ALL REINFORCING STEEL AND EMBEDMENTS SECURELY IN PLACE PRIOR TO PLACING CONCRETE. PROVIDE SUFFICIENT SUPPORTS TO MAINTAIN THE POSITION OF REINFORCEMENT WITHIN SPECIFIED TOLERANCES DURING ALL CONSTRUCTION ACTIVITIES. "STICKING" DOWELS OR REBAR INTO WET CONCRETE IS NOT PERMITTED.

3.8 PROVIDE CONTINUOUS REINFORCEMENT WHEREVER POSSIBLE; SPLICE ONLY AS SHOWN OR APPROVED; STAGGER SPLICES WHERE POSSIBLE; USE CLASS "B" TENSION SPLICE UNLESS NOTED OTHERWISE. DOWELS SHALL MATCH THE SIZE AND SPACING OF THE SPECIFIED REINFORCEMENT AND SHALL BE LAPPED WITH CLASS "B" TENSION SPLICES. UNLESS NOTED OTHERWISE, LAP LENGTHS EXPRESSED AS NUMBER OF BAR DIAMETERS SHALL BE AS FOLLOWS:

BAR SIZE	NORMAL WEIGHT CONCRETE, f'c (PSI)				
	CLASS	3000	4000	5000	6000
#6 OR SMALLER	A	44 DIA.	38 DIA.	34 DIA.	31 DIA.
	B	57 DIA.	50 DIA.	45 DIA.	41 DIA.
#7 OR GREATER	A	55 DIA.	48 DIA.	43 DIA.	39 DIA.
	B	72 DIA.	62 DIA.	56 DIA.	51 DIA.

3.9 REINFORCING STEEL SHALL HAVE THE FOLLOWING CONCRETE COVER UNLESS NOTED OTHERWISE:

CONCRETE CAST AGAINST EARTH (NOT FORMED)	3"
FORMED CONCRETE EXPOSED TO EARTH OR WEATHER:	2"
CONCRETE NOT EXPOSED TO EARTH OR WEATHER:	
SLABS & WALLS	1 1/2"
BEAM STIRRUPS & COLUMN TIES	1 1/2"

3.10 DO NOT PLACE PIPES OR DUCTS EXCEEDING ONE-THIRD THE SLAB OR WALL THICKNESS WITHIN THE SLAB OR WALL UNLESS SPECIFICALLY SHOWN AND DETAILED STRUCTURAL DRAWINGS.

3.11 DO NOT WELD OR TACK WELD REINFORCING STEEL UNLESS APPROVED OR DIRECTED BY THE STRUCTURAL ENGINEER.

3.12 REINFORCING STEEL PLACEMENT SHALL BE INSPECTED IN ACCORDANCE WITH ACI 318, SECTION 1.3 BY A QUALIFIED OWNER'S REPRESENTATIVE.

3.13 NO SLAB SHALL HAVE COLD JOINTS IN A HORIZONTAL PLANE.

3.14 THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE LOCATION AND PLACEMENT OF INSERTS, EMBEDDED PLATES, MASONRY ANCHORS, REGLETS, SLEEVES, DUCTWORK, PADS, AND ANCHOR RODS. THE INSERTS, EMBEDDED PLATES, ETC. SHALL NOT INTERFERE WITH CONCRETE REINFORCEMENT LOCATION.

3.15 NO OPENING SHALL BE MADE IN ANY STRUCTURAL MEMBER WITHOUT THE WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD.

3.16 EXPOSED EXTERNAL CONCRETE CORNERS SHALL BE CHAMFERED 3/4 INCHES, UNLESS SHOWN OR NOTED OTHERWISE.

3.17 ARRANGEMENT AND DETAILS FOR REINFORCEMENT, INCLUDING BAR SUPPORTS AND SPACERS, SHALL BE IN ACCORDANCE WITH THE "A.C.I. DETAILING MANUAL (ACI SP-66)".

STRUCTURAL STEEL

4.0 ALL STRUCTURAL STEEL CONSTRUCTION SHALL CONFORM TO THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION INC. "STEEL CONSTRUCTION MANUAL".

4.1 UNLESS NOTED OTHERWISE, STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING:

STEEL SHAPE (AISC)	TYPE OF STEEL (ASTM)
ANGLES & PLATES	A36
S, M, C, MC	A36 OR A572 GRADE 50
HP	A572 GRADE 50
W	A992
PIPE	A53 GRADE B
HSS	A500 GRADE B OR C
TIE ROD	A36 OR A307A

4.2 ALL BOLT HOLES SHALL BE STANDARD SIZE (BOLT Ø +1/16") UNLESS NOTED OTHERWISE ON DRAWINGS.

4.3 STRUCTURAL STEEL WORK SHALL CONFORM TO THE AISC "ALLOWABLE STRESS DESIGN" SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS," AND THE AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES."

4.4 ANCHOR RODS SHALL BE ASTM F1554, 3/4" DIAMETER WITH 4" HOOKS AND 9" EMBEDMENT, UNLESS NOTED OTHERWISE.

4.5 HIGH STRENGTH BOLTING SHALL BE DONE IN ACCORDANCE WITH AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR ASTM A490 BOLTS."

4.6 BOLTS, NUTS, AND WASHERS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A325. BOLTS SHALL BE 3/4 INCH DIAMETER MINIMUM.

4.7 WELDING SHALL BE DONE BY CERTIFIED WELDERS AND SHALL CONFORM TO AWS D1.1 "STRUCTURAL WELDING CODE-STEEL". ALL WELDING ELECTRODES SHALL BE E70XX.

4.8 THE FABRICATOR/ERECTOR SHALL SUBMIT TO THE ARCHITECT FOR REVIEW, ENGINEERED AND CHECKED DRAWINGS SHOWING SHOP FABRICATION DETAILS, FIELD ASSEMBLY DETAILS, AND ERECTION DIAGRAMS FOR ALL STRUCTURAL STEEL.

4.9 UNLESS NOTED OTHERWISE, CONNECTIONS SHALL BE EITHER AISC SINGLE PLATE OR DOUBLE ANGLE SHEAR CONNECTIONS USING A325 BOLTS IN SHEAR BEARING TYPE CONNECTIONS.

4.10 ALL STRUCTURAL BOLTS SHALL BE SNUG-TIGHTENED JOINTS PER RCSC. ALL BOLTS SHALL BE TIGHTENED SUFFICIENTLY TO PREVENT THE TURNING OF THE NUTS WITHOUT THE USE OF A WRENCH AND CREATE FIRM CONTACT BETWEEN THE CONNECTING ELEMENTS.

4.11 ALL BOLTS SHALL BE INSTALLED PER OF RCSC "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS".

4.12 FIELD CONNECTIONS, EXCEPT WHERE SHOWN TO BE WELDED, SHALL BE BOLTED.

4.13 CONNECTIONS SHALL BE DESIGNED BY STRUCTURAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED FOR THE BEAM REACTION INDICATED ON THE DRAWINGS. CALCULATIONS MUST BE SUBMITTED WITH THE SHOP DRAWING FOR REVIEW AND APPROVAL IN CASES WHERE REACTIONS ARE NOT INDICATED PROVIDE AT LEAST ONE HALF (1/2) OF THE UNIFORM LOAD CARRYING CAPACITY OF BEAM. THE MINIMUM NUMBER OF BOLTS FOR ANY CONNECTIONS SHALL BE TWO (2).

4.14 BEAMS AND JOISTS SHALL BE FABRICATED WITH THE NATURAL CAMBER UP. PROVIDE CAMBERS AS INDICATED ON THE DRAWINGS.

4.15 THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONTROL OF ALL ERECTION PROCEDURES AND SEQUENCES WITH RELATION TO TEMPERATURE DIFFERENTIALS, ESPECIALLY WITH RESPECT TO STRUCTURAL STEEL FRAMING INTO CONCRETE WALLS, BEAMS, OR COLUMNS.

4.16 THERE SHALL BE NO FIELD CUTTING OF STRUCTURAL STEEL MEMBERS FOR THE WORK OF OTHER TRADES WITHOUT THE PRIOR WRITTEN APPROVAL OF THE ARCHITECT.

4.17 ERECT AND MAINTAIN TEMPORARY BRACING TO INSURE THE ALIGNMENT AND STABILITY OF THE STRUCTURE DURING ERECTION UNTIL PERMANENT CONNECTIONS HAVE BEEN COMPLETED.

4.18 ALL STRUCTURAL STEEL EXPOSED TO VIEW IN THE FINISHED BUILDING SHALL CONFORM TO THE AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES, "SECTION 10, "ARCHITECTURAL EXPOSED STRUCTURAL STEEL".

4.19 ALL EXTERIOR STRUCTURAL STEEL SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A123 AND ASTM A386. FIELD WELDS SHALL BE GROUND SMOOTH AND TOUCH UP IN ACCORDANCE WITH ASTM A780.

4.20 ALL STAIRWAYS, PLATFORMS AND LANDINGS SHALL BE ADEQUATE TO SUPPORT A LIVE LOAD OF 100 POUNDS PER SQUARE FOOT AND A CONCENTRATED LOAD OF 300 POUNDS. THE FABRICATOR/ELECTOR SHALL SUBMIT TO THE ARCHITECT FOR REVIEW, ENGINEERED AND CHECKED DRAWING SHOWING SHOP FABRICATION DETAILS, AND ERECTION DIAGRAMS FOR ALL STAIRWAYS PLATFORMS AND LANDINGS. THE CONTRACTOR SHALL SUBMIT CERTIFICATE OF APPROVAL OF THE STEEL FABRICATOR AS DEFERRED SUBMITTAL.

PREFABRICATED WOOD TRUSSES

5.0 PREFABRICATED WOOD TRUSSES SHALL COMPLY WITH NDS AND "NATIONAL DESIGN STANDARD FOR METAL PLATE CONNECTED WOOD TRUSS CONSTRUCTION (TPI 1-14)".

5.1 ALL TRUSSES SHALL BE DESIGNED BY THE TRUSS MANUFACTURER'S REGISTERED ENGINEER.

5.2 TRUSSES SHALL BE FABRICATED BASED ON VERIFIED OR APPROVED DIMENSIONS.

5.3 FABRICATOR SHALL SUBMIT SHOP DRAWINGS AND TRUSS PLACEMENT PLANS(S) FOR ENGINEER APPROVAL PRIOR TO FABRICATION. TRUSS FABRICATOR SHALL SUBMIT CALCULATIONS TO ENGINEER OF RECORD FOR REVIEW AND APPROVAL PRIOR TO FABRICATION.

5.4 JOBSITE HANDLING AND ERECTION, INCLUDING TEMPORARY BRACING, DURING INSTALLATION OF TRUSSES SHALL COMPLY WITH TPIWTCA BCSI 1.

5.5 UNLESS OTHERWISE NOTED, DESIGN TRUSSES TO MEET THE FOLLOWING DEFLECTION CRITERIA:

	TOTAL LOAD	LIVE LOAD
FLOOR TRUSS:	SPAN/240	SPAN/480
ROOF TRUSS:	SPAN/240	SPAN/360

5.6 PERMANENT INDIVIDUAL TRUSS MEMBER RESTRAINT/BRACING SHALL BE INSTALLED USING STANDARD INDUSTRY LATERAL RESTAINT/BRACING DETAILS IN ACCORDANCE WITH GENERALLY ACCEPTED ENGINEERING PRACTICE. LOCATIONS AND DETAILS FOR LATERAL RESTRAINT SHALL BE IDENTIFIED ON THE TRUSS DESIGN DRAWINGS.

5.7 MINIMUM WOOD TRUSS MEMBER SIZES SHALL BE 2x4 (NOMINAL).

5.8 STRONG BACK SHALL BE PROVIDED AND SPACED NOT MORE THAN 10'-0" TO MINIMIZE VIBRATION.

DRAWING LIST

ABBREVIATIONS

&	=	AND
@	=	AT
A.	=	ALTERNATE
APPROX.	=	APPROXIMATE
BLDG.	=	BUILDING
BM.	=	BEAM
BRG.	=	BEARING
BSMT.	=	BASEMENT
BTWN.	=	BETWEEN
B/	=	BOTTOM OF
BOT.	=	BOTTOM
BLKG.	=	BLOCKING
BLK.	=	BLOCK
CFMF	=	COLD-FORMED METAL FRAMING
CFS	=	COLD-FORMED STEEL
C.J.	=	CONTROL OR CONSTRUCTION JOINT
℄	=	CENTER LINE
CLG.	=	CEILING
CMU	=	CONC. MASONRY UNIT
COL.	=	COLUMN
CONC.	=	CONCRETE
CONT.	=	CONTINUOUS
CONST.	=	CONSTRUCTION
DTL.	=	DETAIL
DIA.	=	DIAMETER
DIM.	=	DIMENSION
DN.	=	DOWN
DR.	=	DOOR
DWG.	=	DRAWING
DBL.	=	DOUBLE
EA.	=	EACH
E.F.	=	EACH FACE
EL.	=	ELEVATION
EQ.	=	EQUAL
EQP.	=	EQUIPMENT
E.W.	=	EACH WAY
EX.	=	EXISTING
EXT.	=	EXTERIOR
EXP.	=	EXPANSION
F.D.	=	FLOOR DRAIN
F.F.	=	FINISH FLOOR
FL.	=	FLOOR
FDN.	=	FOUNDATION
F.O.	=	FACE OF
F.S.	=	FAR SIDE
FT.	=	FOOT
FTG.	=	FOOTING
GA.	=	GAUGE
GALV.	=	GALVANIZED
GYP.BD.	=	GYPSUM BOARD
HDR.	=	HEADER
HORIZ.	=	HORIZONTAL
HGT.	=	HEIGHT
I.D.	=	INSIDE DIAMETER
INSUL.	=	INSULATION
INT.	=	INTERIOR
JT.	=	JOINT
LG	=	LONG
LLV	=	LONG LEG VERTICAL
LT.	=	LIGHT
L.W.	=	LONG WAY
MANUF.	=	MANUFACTURER
MAS.	=	MASONRY
MAX.	=	MAXIMUM
MECH.	=	MECHANICAL
MTL.	=	METAL
MIN.	=	MINIMUM
MISC.	=	MISCELANEOUS
N.I.C.	=	NOT IN CONTRACT
NO.	=	NUMBER
NOM.	=	NOMINAL
N.T.S.	=	NOT TO SCALE
O.C.	=	ON CENTER
O.D.	=	OUTSIDE DIAMETER
OPNG.	=	OPENING
OPT.	=	OPTIONAL
P.C.	=	PRECAST CONCRETE
PERP.	=	PERPENDICULAR
PL.	=	PLATE
PLYWD.	=	PLYWOOD
PT	=	PRESSURE-TREATED
QTY.	=	QUANTITY
RAD.	=	RADIUS
REF.	=	REFERENCE
REINF.	=	REINFORCEMENT
REV.	=	REVISION
REQ'D.	=	REQUIRED
R.O.	=	ROUGH OPENING
SCHED.	=	SCHEDULE
SEC.	=	SECTION
S.F.	=	SQUARE FOOT
SHT.	=	SHEET
SIM.	=	SIMILAR
SLV	=	SHORT LEG VERTICAL
SPEC.	=	SPECIFICATION
SQ	=	SQUARE
S.S.	=	STAINLESS STEEL
STD.	=	STANDARD
STL.	=	STEEL
STRUCT.	=	STRUCTURAL
S.W.	=	SHORT WAY
T&B	=	TOP & BOTTOM
THK.	=	THICK
THRU	=	THROUGH
T/	=	TOP OF
T.O.C.	=	TOP OF CONCRETE
T.O.F.	=	TOP OF FOOTING
T.O.P.	=	TOP OF PLATE
T.O.S.	=	TOP OF STEEL
T.O.SL.	=	TOP OF SLAB
TRT.	=	TREATED
TYP.	=	TYPICAL
U.N.O.	=	UNLESS NOTED OTHERWISE
VERT.	=	VERTICAL
V.I.F.	=	VERIFY IN FIELD
W/	=	WITH
WD	=	WOOD
W/O	=	WITHOUT
WP	=	WORK POINT
WP.	=	WATERPROOF
WT	=	WEIGHT
WWF.	=	WELDED WIRE FABRIC
WWM.	=	WELDED WIRE MESH
WWR.	=	WELDED WIRE REINFORCEMENT

OFFICIAL SEAL

CLIENT

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CONSULTANT

TROPICAL SMOOTHIE CAFE

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WEST LAFAYETTE, IN 47906

REV	REV DESCRIPTION	DATE
	ISSUED FOR PERMIT	04/27/26

KEVIN J PATEL
REGISTERED
No. PE11800241
STATE OF
INDIANA
PROFESSIONAL ENGINEER
EXP: 07/31/2026
DATE: 06/17/2026

DESIGN PROFESSIONAL SEAL

Drawn / Designed by:	NA/KV
Reviewed by:	RU
Approved by:	KP
Project Number:	**KPJ-25-0282**

SHEET NAME:

GENERAL NOTES

SHEET NUMBER:

S-101

WOOD NOTES

6.0 UNLESS NOTED OTHERWISE, ALL WOOD CONSTRUCTION SHALL CONFORM TO CONVENTIONAL LIGHT-FRAME CONSTRUCTION PER THE BUILDING CODE AS LISTED IN 1.0.

6.1 UNLESS NOTED OTHERWISE, ALL SOLID SAWN LUMBER SHALL BE KILN DRIED GRADE NO. 2 DOUGLAS FIR-LARCH (OR BETTER). WOOD MOISTURE CONTENT MUST BE <19% WITH THE FOLLOWING MINIMUM PROPERTIES:

E	=	1,600,000 PSI
F_b	=	900 PSI
F_v	=	180 PSI
F_c (PARALLEL)	=	1,350 PSI
F_c (PERP.)	=	625 PSI
F_t	=	575 PSI

6.2 UNLESS NOTED OTHERWISE, ENGINEERED LUMBER MEMBERS SHALL BE MANUFACTURED UNDER A PROCESS BY THE NATIONAL RESEARCH BOARD. EACH PIECE SHALL BEAR A STAMPS OR STAMPS NOTING THE NAME AND PLANT NUMBER OF THE MANUFACTURER, THE GRADE, THE NATIONAL RESEARCH BOARD NUMBER, AND THE QUALITY CONTROL AGENCY. ALL LUMBER SHALL BE MANUFACTURED IN ACCORDANCE WITH THE APPROPRIATE REPORT AND GLUED USING A WATERPROOFING ADHESIVE MEETING THE REQUIREMENTS OF ASTM D2559 WITH ALL GRAIN PARALLEL WITH THE LENGTH OF THE MEMBER WITH THE FOLLOWING MINIMUM PROPERTIES:

LAMINATED VENEER LUMBER (LVL)

E	=	2,000,000 PSI
F_b	=	2,600 PSI
F_v	=	285 PSI
F_c (PARALLEL)	=	2,510 PSI
F_c (PERP.)	=	750 PSI
F_t	=	1,555 PSI

PARALLAM (PSL) LUMBER-COLUMN

E	=	2,000,000 PSI
F_b	=	2,900 PSI
F_v	=	290 PSI
F_c (PARALLEL)	=	2,900 PSI
F_c (PERP.)	=	625 PSI
F_t	=	2,025 PSI

6.3 PRESSURE TREATED LUMBER SHALL CONFORM TO THE AWPA AND SHALL BEAR THE QUALITY MARK OF AN ACCREDITED ALSC INSPECTION AGENCY. MINIMUM TREATING STANDARDS (RETENTION LBS./CU. FT) SHALL BE AS FOLLOWS:

APPLICATION	ACQ/ACZA	CA-B
ABOVE GROUND	0.25	0.10
GROUND/CONCRETE CONTACT	0.40	0.21
FRESH WATER IMMERSION	0.40	0.21
IN GROUND (STRUCTURAL)	0.60	0.31
SILL PLATES	0.25	0.10

6.4 WOOD SHALL BE UNWRAPPED & FREE OF SPLITS & CHECKS.

6.5 ALL DIMENSION LUMBER EXPOSED TO WEATHER MUST BE WEATHER TREATED PER AMERICAN WOOD PRESERVERS ASSOCIATION. PARALLEL STRAND LUMBER (PSL), SHALL BE WEATHER TREATED. TREATMENT MUST BE CURRENTLY ACCEPTED, ENVIRONMENTALLY FRIENDLY TREATMENT; CCA TREATMENT IS NOT PERMITTED. ALL METAL FASTENERS EXPOSED TO WEATHER, INCLUDING BUT NOT LIMITED TO STRAPS, HANGERS, BOLTS, SCREWS, NAILS AND OTHER SPECIAL FASTENING HARDWARE TO BE CORROSIVE RESISTANT. PROCESS MUST BE HOT DIP GALVANIZED ZINC COATING WITH MINIMUM 1.85 OZ OF ZINC COATING PER SQUARE FOOT.

6.6 UNLESS NOTED OTHERWISE, ALL POSTS SHALL BE MINIMUM 4X4, ALL POSTS SUPPORTING HEADERS AT MAXIMUM OPENING 4'-0" SHALL BE MINIMUM SIZE 2-2X4, AND ALL HEADERS SHALL BE MINIMUM SIZE 2-2X10.

6.7 ALL COLUMNS ARE TO BE CONTINUOUS TO THE FOUNDATION OR TO THE SUPPORTING BEAM. TRANSFER MEMBER IS PERMITTED IF SPECIFICALLY ENGINEERED FOR THIS PURPOSE.

6.8 UNLESS NOTED OTHERWISE, SIMPSON COLUMN CAPS AC OR SIMILAR USP SHALL BE USED AT ALL CONNECTIONS OF LVL BEAMS TO PSL COLUMNS.

6.9 DUE TO CORROSION REQUIREMENTS, ALL FASTENERS USED IN OR IN CONTACT WITH PRESERVATIVE-TREATED WOOD SHALL BE HOT-DIPPED GALVANIZED STEEL OR STAINLESS STEEL. OTHER TYPES OF PROTECTIVE FASTENER COATINGS MAY BE USED UPON ENGINEERING REVIEW AND APPROVAL.

6.10 UNLESS NOTED OTHERWISE, ALL LOAD-BEARING WALL STUDS SHALL BE 2x6 SOLID SAWN LUMBER WITH A MAXIMUM SPACING OF 16" O.C.

6.11 USE THREE STUDS (MINIMUM) IN EACH CORNER WITH TOP PLATES OVERLAPPING AT CORNERS AND BEARING WALLS. SILL PLATES OR ANY OTHER WOOD IN CONTACT WITH CONCRETE, SOIL OR MASONRY SHALL BE PRESSURE TREATED.

6.12 BUILT-UP STUD COLUMNS MUST BE SECURELY NAILED TOGETHER TO ACT AS A COMPOSITE MEMBER.

6.13 UNLESS NOTED OTHERWISE, ALL LOAD BEARING WALLS SHALL BE SHEATHED WITH MIN. 7/16" APA RATED SHEATHING, EXPOSURE I, OSB OR PLYWOOD. WALL SHEATHING SHALL BE FASTENED WITH 8d COMMON NAILS WITH A MAXIMUM SPACING OF 6" O.C. AT EDGES AND 12" O.C. WITHIN THE FIELD. A DOUBLE ROW OF 8d COMMON NAILS WITH A MAXIMUM SPACING OF 6" O.C. SHALL BE USED AT TOP AND BOTTOM PANEL EDGES FOR UPLIFT RESISTANCE. ALL EDGES OF SHEATHING SHALL BE BLOCKED SOLID. CONTINUOUS SHEATHING SHALL BE PROVIDED FROM TOP PLATE TO SILL PLATE. USE APPROVED SHEATHING PANEL TENSION SPLICE AS REQUIRED.

6.14 UNLESS NOTED OTHERWISE, ROOF SHEATHING SHALL BE APA RATED SHEATHING, EXPOSURE I, FOR ROOF SUPPORTS AT 24" O.C. USE 15/32" OSB OR PLYWOOD. FOR ROOF SUPPORTS @ 16" O.C. USE 7/16" OSB OR PLYWOOD. SHEATHING SHALL BE INSTALLED WITH THE LONG EDGES ACROSS A MINIMUM OF THREE SUPPORTING MEMBERS. SUPPORT AND STAGGER EDGES OF SHEATHING PARALLEL TO SUPPORTING MEMBERS. ROOF SHEATHING SHALL BE FASTENED WITH 8d COMMON NAILS WITH A MAXIMUM SPACING OF 6" O.C. AT EDGES AND 12" O.C. WITHIN THE FIELD. PANEL CLIPS SHALL BE INSTALLED IN ACCORDANCE WITH APA RECOMMENDATIONS.

6.15 UNLESS NOTED OTHERWISE, FLOOR SHEATHING SHALL BE APA RATED STURD-I-FLOOR, EXPOSURE I, TONGUE AND GROOVE PLYWOOD. FOR SUPPORTS AT 16" O.C. USE 19/32" THICK PANELS. FASTEN FLOOR SHEATHING WITH 10d RING-OR SCREW-SHANK NAILS WITH A MAXIMUM SPACING OF 6" O.C. AT EDGES AND 12" O.C. WITHIN THE FIELD. PANELS SHALL BE APA GLUED TO FLOOR SUPPORTS AND AT TONGUE AND GROOVE JOINTS WITH ADHESIVE CONFORMING TO PERFORMANCE SPECIFICATION APA AFG-01. AN 1/8" GAP SHALL BE LEFT BETWEEN ADJACENT PANELS AND THE END JOINTS SHALL BE SUPPORTED AND STAGGERED. SHEATHING SHALL BE INSTALLED WITH THE LONG EDGE ACROSS A MINIMUM OF THREE SUPPORTING MEMBERS. PROVIDE CONTINUOUS BLOCKING AT EDGES AND AROUND OPENINGS.

6.16 UNLESS NOTED OTHERWISE, DOOR AND WINDOW HEADERS SHALL FULLY BEAR ON JACK STUDS. KING STUDS SHALL EXTEND THE FULL HEIGHT OF WALL ON EACH SIDE OF OPENING.

6.17 BEARING & EXTERIOR WALL STUDS SHALL BE CAPPED WITH DOUBLE TOP PLATES INSTALLED TO PROVIDE OVERLAPPING AT CORNERS AND INTERSECTIONS WITH OTHER PARTITIONS. END JOINTS IN DOUBLE TOP PLATES SHALL BE OFFSET AT LEAST 48" & SHALL BE NAILED WITH NOT LESS THAN (16) 16d FACE NAILS ON EACH SIDE OF JOINT. TOP PLATES SHALL BE A NOMINAL 2" DEPTH AND HAVE A WIDTH AT LEAST EQUAL TO THE WIDTH OF THE STUDS.

6.18 SOLID 2x BLOCKING AND BANDBOARD SHALL BE PROVIDED AT SUPPORTS AND CANTILEVER ENDS OF ALL WOOD JOISTS.

6.19 ALL MANUFACTURED HARDWARE SHALL BE INSTALLED IN ACCORDANCE WITH THE APPLICABLE MANUFACTURER'S SPECIFICATIONS.

6.20 GYPSUM WALLBOARD SHALL BE SECURED WITH A MAXIMUM FASTENER SPACING OF 6" O.C. AT EDGES AND 12" O.C. AT FIELD.

WOOD FASTENING SCHEDULE U.N.O.

CONNECTION	FASTENING	LOCATION
JOIST TO SILL OR GIRDER	3 - 8D COMMON (2 1/2" × 0.131") 3 - 3" × 0.131" NAILS	TOE NAIL
BRIDGING TO JOIST	2 - 8D COMMON (2 1/2" × 0.131") 2 - 3" × 0.131" NAILS	TOE NAIL EACH END
1"x 6" SUBFLOOR OR LESS TO EACH JOIST	2 - 8D COMMON (2 1/2" × 0.131")	FACE NAIL
WIDER THAN 1"x 6" SUBFLOOR TO EACH JOIST	3 - 8D COMMON (2 1/2" × 0.131")	FACE NAIL
2" SUBFLOOR TO JOIST OR GIRDER	2 - 16D COMMON (3 1/2" × 0.162")	BLIND AND FACE NAIL
SOLE PLATE TO JOIST OR BLOCKING	16D (3 1/2" × 0.135") AT 16" O.C. 3" × 0.131" NAILS AT 8" O.C.	TYPICAL FACE NAIL
SOLE PLATE TO JOIST OR BLOCKING AT BRACED WALL PANEL	3 - 16D (3 1/2" × 0.135") @ 16" O.C. 4 - 3" × 0.131" NAILS AT 16" O.C.	BRACED WALL PANELS
TOP PLATE TO STUD	2 - 16D COMMON (3 1/2" × 0.162") 3 - 3" × 0.131" NAILS	END NAIL
STUD TO SOLE PLATE	4 - 8D COMMON (2 1/2" × 0.131") 4 - 3" × 0.131" NAILS	TOE NAIL
	2 - 16D COMMON (3 1/2" × 0.162") 3 - 3" × 0.131" NAILS	END NAIL
DOUBLE STUDS	16D (3 1/2" × 0.135") AT 24" O.C. 3" × 0.131" NAIL AT 8" O.C.	FACE NAIL
DOUBLE TOP PLATES	3 - 8D COMMON (2 1/2" × 0.131") 3 - 3" × 0.131" NAILS	TYPICAL FACE NAIL
BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE	3 - 8D COMMON (2 1/2" × 0.131") 3 - 3" × 0.131" NAILS	TOE NAIL
RIM JOIST TO TOP PLATE	8D (2 1/2" × 0.131") AT 6" O.C. 3" × 0.131" NAIL AT 6" O.C.	TOE NAIL
TOP PLATES, LAPS AND INTERSECTIONS	2 - 16D COMMON (3 1/2" × 0.162") 3 - 3" × 0.131" NAILS	FACE NAIL
CONTINUOUS HEADER, TWO PIECES	16D COMMON (3 1/2" × 0.162")	16" O.C. ALONG EDGE
CEILING JOISTS TO PLATE	3 - 8D COMMON (2 1/2" × 0.131") 5 - 3" × 0.131" NAILS	TOE NAIL
CONTINUOUS HEADER TO STUD	4 - 8D COMMON (2 1/2" × 0.131") 3 - 16D COMMON (3 1/2" × 0.162")MINIMUM,	TOE NAIL
CEILING JOISTS, LAPS OVER PARTITIONS	4 - 3" × 0.131" NAILS 3 - 16D COMMON (3 1/2" × 0.162") MINIMUM,	FACE NAIL
CEILING JOISTS TO PARALLEL RAFTERS	4 - 3" × 0.131" NAILS	FACE NAIL
RAFTER TO PLATE	3 - 8D COMMON (2 1/2" × 0.131") 3 - 3" × 0.131" NAILS	TOE NAIL
1" DIAGONAL BRACE TO EACH STUD AND PLATE	2 - 8D COMMON (2 1/2" × 0.131") 2 - 3" × 0.131" NAILS	FACE NAIL
1" × 8" SHEATHING TO EACH BEARING	3 - 8D COMMON (2 1/2" × 0.131")	FACE NAIL
WIDER THAN 1" × 8" SHEATHING TO EACH BEARING	3 - 8D COMMON (2 1/2" × 0.131")	FACE NAIL
2" PLANKS	16D COMMON (3 1/2" × 0.162")	AT EACH BEARING
COLLAR TIE TO RAFTER	3 - 10D COMMON (3" × 0.148") 4 - 3" × 0.131" NAILS	FACE NAIL
JACK RAFTER TO HIP	3 - 10D COMMON (3" × 0.148") 4 - 3" × 0.131" NAILS	TOE NAIL
	2 -16D COMMON (3 1/2" × 0.162") 3 - 3" × 0.131" NAILS	FACE NAIL
ROOF RAFTER TO 2-BY RIDGE BEAM	2 -16D COMMON (3 1/2" × 0.162") 3 - 3" × 0.131" NAILS	TOE NAIL
	2 -16D COMMON (3 1/2" × 0.162") 3 - 3" × 0.131" NAILS	FACE NAIL
JOIST TO BAND JOIST	3 - 16D COMMON (3 1/2" × 0.162") 4 - 3" × 0.131" NAILS	FACE NAIL
LEDGER STRIP	3 - 16D COMMON (3 1/2" × 0.162") 4 - 3" × 0.131" NAILS	FACE NAIL AT EACH END

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INDIANA
PROFESSIONAL ENGINEER

EXP: 07/31/2026
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Drawn / Designed by:	NA/KV
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GENERAL NOTES

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S-102

GENERAL

1. ALL SPECIAL INSPECTIONS SHALL BE PERFORMED IN ACCORDANCE WITH CHAPTER 17 OF THE BUILDING CODE AS LISTED IN 1.0. INCLUDING ADOPTED AMENDMENTS. SPECIAL INSPECTIONS ARE IN ADDITION TO INSPECTIONS REQUIRED BY SECTION 110 OF THE BUILDING CODE AS LISTED IN 1.0.

2. THE SPECIAL INSPECTOR SHALL OBSERVE THE INDICATED WORK FOR COMPLIANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS. ALL DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE CONTRACTOR FOR CORRECTION AND NOTED IN THE INSPECTION REPORTS.

3. THE SPECIAL INSPECTOR AND GEOTECHNICAL ENGINEER SHALL FURNISH INSPECTION REPORTS FOR EACH INSPECTION TO THE BUILDING OFFICIAL, STRUCTURAL ENGINEER, ARCHITECT, CONTRACTOR, AND OWNER. THE SPECIAL INSPECTION AGENCY SHALL SUBMIT A FINAL REPORT STATING THAT THE WORK REQUIRING SPECIAL INSPECTION WAS INSPECTED AND IS IN CONFORMANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS AND THAT ALL DISCREPANCIES NOTED IN THE INSPECTION REPORTS HAVE BEEN CORRECTED.

4. INSPECTION TYPES

CONTINUOUS :	THE FULL-TIME OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED.
PERIODIC :	THE PART-TIME OR INTERMITTENT OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK HAS BEEN OR IS BEING PERFORMED AND AT THE COMPLETION OF THE WORK.
OBSERVE :	OBSERVE THESE FUNCTIONS ON A RANDOM, DAILY BASIS. OPERATIONS NEED NOT BE DELAYED PENDING OBSERVATIONS.
PERFORM :	INSPECTIONS SHALL BE PERFORMED PRIOR TO THE FINAL ACCEPTANCE OF THE ITEM.

MATERIALS

1. NEW MATERIALS: NEW BUILDING MATERIALS, EQUIPMENT, APPLIANCES, SYSTEMS OR METHODS OF CONSTRUCTION NOT PROVIDED FOR IN THIS CODE, AND ANY MATERIAL QUESTIONED SUITABILITY PROPOSED FOR USE IN THE CONSTRUCTION OF A BUILDING OR STRUCTURE, SHALL BE SUBJECTED TO THE TESTS PRESCRIBED IN THIS CHAPTER AND IN THE APPROVED RULES TO DETERMINE CHARACTER, QUALITY AND LIMITATIONS OF USE.

2. USED MATERIALS: THE USE OF SECOND HAND MATERIALS THAT MEET THE MINIMUM REQUIREMENTS OF THE BUILDING CODE AS LISTED IN 1.0. FOR NEW MATERIALS SHALL BE PERMITTED.

APPROVALS

1. APPROVED AGENCY: AN APPROVED AGENCY SHALL PROVIDE ALL INFORMATION AS NECESSARY FOR THE BUILDING OFFICIAL TO DETERMINE THAT THE AGENCY MEETS THE APPLICABLE REQUIREMENTS SPECIFIED IN SECTIONS 1703.1.1 THROUGH 1703.1.3 OF THE BUILDING CODE AS LISTED IN 1.0.

   A. INDEPENDENCE: AN APPROVED AGENCY SHALL BE OBJECTIVE, COMPETENT AND INDEPENDENT FROM THE CONTRACTOR RESPONSIBLE FOR THE WORK BEING INSPECTED. THE AGENCY SHALL DISCLOSE TO THE BUILDING OFFICIAL AND THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE POSSIBLE CONFLICTS OF INTEREST SO THAT OBJECTIVITY CAN BE CONFIRMED.

   B. EQUIPMENT: AN APPROVED AGENCY SHALL HAVE ADEQUATE EQUIPMENT TO PERFORM REQUIRED TESTS. THE EQUIPMENT SHALL BE PERIODICALLY CALIBRATED.

   C. PERSONNEL: AN APPROVED AGENCY SHALL EMPLOY EXPERIENCED PERSONNEL EDUCATED IN CONDUCTING, SUPERVISING, AND EVALUATING TESTS, AND SPECIFICAL INSPECTIONS.

2. WRITTEN APPROVAL: ANY MATERIAL, APPLIANCE, EQUIPMENT, SYSTEM OR METHOD OF CONSTRUCTION MEETING THE REQUIREMENTS OF THIS CODE SHALL BE APPROVED IN WRITING AFTER SATISFACTORY COMPLETION OF THE REQUIRED TESTS AND SUBMISSION OF REQUIRED TEST REPORTS.

3. RECORD OF APPROVAL: FOR ANY MATERIAL, APPLIANCE, EQUIPMENT, SYSTEM OR METHOD OF CONSTRUCTION THAT HAS BEEN APPROVED, A RECORD OF SUCH APPROVAL, INCLUDING THE CONDITIONS AND LIMITATIONS OF THE APPROVAL , SHALL BE KEPT ON FILE IN THE BUILDING OFFICIAL'S OFFICE AND SHALL BE AVAILBLE FOR PUBLIC REVIEW AT APPROPRIATE TIMES.

4. PERFORMANCE: SPECIFIC INFORMATION CONSISTING OF TEST REPORTS CONDUCTED BY AN APPROVED AGENCY IN ACCORDANCE WITH THE APPROPRIATE REFERENCED STANDARDS, OR OTHER SUCH INFORMATION AS NECESSARY, SHALL BE PROVIDED FOR THE BUILDING OFFICIAL TO DETERMINE THAT THE PRODUCT, MATERIAL OR ASSEMBLY MEETS THE APPLICABLE CODE REQUIREMENTS.

   A. RESEARCH AND INVESTIGATION: SUFFICIENT TECHNICAL DATA SHALL BE SUBMITTED TO THE BUILDING OFFICIAL TO SUBSTANTIATE THE PROPOSED USE OF ANY PRODUCT, MATERIAL OR ASSEMBLY. IF IT IS DETERMINED THAT THE EVIDENCE SUBMITTED IS SATISFACTORY PROOF OF PERFORMANCE FOR THE USE INTENDED,THE BUILDING OFFICIAL SHALL APPROVE THE USE OF THE PRODUCT, MATERIAL OR ASSEMBLY SUBJECT TO THE REQUIREMENTS OF THIS CODE. THE COST REPORTS AND INVESTIGATIONS REQUIRED UNDER THESE PROVISIONS SHALL BE PAID BY THE OWNER OR THE OWNER'S AUTHORIZED AGENT.

   B. RESEARCH REPORTS: SUPPORTING DATA, WHERE NECESSARY TO ASSIST IN THE APPROVAL OF PRODUCTS, MATERIALS OR ASSEMBLIES NOT SPECIFICALLY PROVIDED FOR IN THIS CODE, SHALL CONSIST OF VALID RESEARCH REPORTS FROM APPROVED SOURCES.

5. LABELING: PRODUCTS, MATERIALS OR ASSEMBLIES REQUIRED TO BE LABELED IN ACCORDANCE WITH THE PROCEDURES SET FORTH IN SECTIONS 1703.5.1 THROUGH 1703.5.4 OF THE BUILDING CODE AS LISTED IN 1.0.

   A. TESTING: AN APPROVED AGENCY SHALL TEST A REPRESENTATIVE SAMPLE OF THE PRODUCT, MATERIAL OR ASSEMBLY BEING LABELED TO THE RELEVANT STANDARD OR STANDARDS. THE APPROVED AGENCY SHALL MAINTAIN A RECORD OF THE TESTS PERFORMED. THE RECORD SHALL PROVIDE SUFFICIENT DETAIL TO VERIFY COMPLIANCE WITH THE TEST STANDARD.

   B. INSPECTION AND IDENTIFICATION: THE APPROVED AGENCY SHALL PERIODICALLY PERFORM AN INSPECTION, WHICH SHALL BE IN-PLANT IF NECESSARY, OF THE PRODUCT OR MATERIAL THAT IS TO BE LABELED. THE INSPECTION SHALL VERIFY THAT THE LABELED PRODUCT, MATERIAL OR ASSEMBLY IS REPRESENTATIVE OF THE PRODUCT, MATERIAL OR ASSEMBLY TESTED.

   C. LABEL INFORMATION: THE LABEL SHALL CONTAIN THE MANUFACTURER'S IDENTIFICATION, MODEL NUMBER, SERIAL NUMBER OR DEFINITIVE INFORMATION DESCRIBING THE PERFORMANCE CHARACTERISTICS OF THE PRODUCT, MATERIAL OR ASSEMBLY AND THE APPROVED AGENCY'S IDENTIFICATION.

   D. METHOD OF LABELING: INFORMATION REQUIRED TO BE PERMANENTLY IDENTIFIED ON THE PRODUCT, MATERIAL OR ASSEMBLY SHALL BE ACID ETCHED, SAND BLASTED, CERAMIC FIRED, LASER ETCHED, EMBOSSED OR OF A TYPE THAT, ONCE APPLIED, CANNOT BE REMOVED WITHOUT BEING DESTROYED.

6. EVALUATION AND FOLLOW-UP INSPECTION SERVICES: WHERE STRUCTURAL COMPONENTS OR OTHER ITEMS REGULATED BY THIS CODE ARE NOT VISIBLE FOR INSPECTION AFTER COMPLETION OF A PREFABRICATED ASSEMBLY, THE OWNER OR THE OWNER'S AUTHORIZED AGENT SHALL SUBMIT A REPORT OF EACH PREFABRICATED ASSEMBLY. THE REPORT SHALL INDICATE THE COMPLETE DETAILS OF THE ASSEMBLY, INCLUDING A DESCRIPTION OF THE ASSEMBLY AND ITS COMPONENTS, THE BASIS UPON WHICH THE ASSEMBLY IS BEING EVALUATED, TEST RESULTS AND SIMILAR INFORMATION AND OTHER DATA AS NECESSARY FOR THE BUILDING OFFICIAL TO DETERMINE CONFORMANCE TO THIS CODE. SUCH A REPORT SHALL BE APPROVED BY THE BUILDING OFFICIAL.

   A. FOLLOW-UP INSPECTION: THE OWNER OR THE OWNER'S AUTHORIZED AGENT SHALL PROVIDE FOR SPECIAL INSPECTIONS OF FABRICATED ITEMS IN ACCORDANCE WITH SECTION 1704.2.5 OF THE BUILDING CODE AS LISTED IN 1.0.

   B. TEST AND INSPECTION RECORDS: COPIES OF NECESSARY TEST AND SPECIAL INSPECTION RECORDS SHALL BE FILED WITH THE BUILDING OFFICIAL.

SPECIAL INSPECTIONS AND TESTS, CONTRACTOR RESPONSIBILITY AND STRUCTURAL OBSERVATION

1. GENERAL: SPECIAL INSPECTIONS AND TESTS, STATEMENTS OF SPECIAL INSPECTIONS, RESPONSIBILITIES OF CONTRACTORS, SUBMITTALS TO THE BUILDING OFFICIAL AND STRUCTURAL OBSERVATIONS SHALL MEET THE APPLICABLE REQUIREMENTS OF THIS SECTION.

2. SPECIAL INSPECTIONS AND TESTS: WHERE APPLICATION IS MADE TO THE BUILDING OFFICIAL FOR CONSTRUCTION AS SPECIFIED IN SECTION 105 OF THE BUILDING CODE AS LISTED IN 1.0, THE OWNER OR THE OWNER'S AUTHORIZED AGENT, OTHER THAN THE CONTRACTOR, SHALL EMPLOY ONE OR MORE APPROVED AGENCIES TO PROVIDE SPECIAL INSPECTIONS AND TESTS DURING CONSTRUCTION ON THE TYPES OF WORK SPECIFIED IN SECTION 1705 OF THE BUILDING CODE AS LISTED IN 1.0 AND IDENTIFY THE APPROVED AGENCIES TO THE BUILDING OFFICIAL. THESE SPECIAL INSPECTIONS AND TESTS ARE IN ADDITION TO THE INSPECTIONS BY THE BUILDING OFFICIAL THAT ARE IDENTIFIED IN SECTION 110 OF THE BUILDING CODE AS LISTED IN 1.0.

   EXCEPTIONS:

   - SPECIAL INSPECTIONS AND TESTS ARE NOT REQUIRED FOR CONSTRUCTION OF A MINOR NATURE OR AS WARRANTED BY CONDITIONS IN THE JURISDICTION AS APPROVED BY THE BUILDING OFFICIAL.
   - UNLESS OTHERWISE REQUIRED BY THE BUILDING OFFICIAL, SPECIAL INSPECTIONS AND TESTS ARE NOT REQUIRED FOR GROUP U OCCUPANCIES THAT ARE ACCESSORY TO RESIDENTIAL OCCUPANCY INCLUDING, BUT NOT LIMITED TO, THOSE LISTED IN SECTION 312.1 OF THE BUILDING CODE AS LISTED IN 1.0.
   - SPECIAL INSPECTIONS AND TESTS ARE NOT REQUIRED FOR PORTIONS OF STRUCTURES DESIGNED AND CONSTRUCTED IN ACCORDANCE WITH THE COLD-FORMED STEEL LIGHT-FRAME CONSTRUCTION PROVISIONS OF SECTION 2211.1.2 OF THE BUILDING CODE AS LISTED IN 1.0 OR THE CONVENTIONAL LIGHT-FRAME CONSTRUCTION PROVISIONS OF SECTION 2308 OF THE BUILDING CODE AS LISTED IN 1.0.
   - THE CONTRACTOR IS PERMITTED TO EMPLOY THE APPROVED AGENCIES WHERE THE CONTRACTOR IS ALSO THE OWNER.

   A. SPECIAL INSPECTOR QUALIFICATIONS: PRIOR TO THE START OF THE CONSTRUCTION, THE APPROVED AGENCIES SHALL PROVIDE WRITTEN DOCUMENTATION TO THE BUILDING OFFICIAL DEMONSTRATING THE COMPETENCE AND RELEVANT EXPERIENCE OR TRAINING OF THE SPECIAL INSPECTORS WHO WILL PERFORM THE SPECIAL INSPECTIONS AND TESTS DURING CONSTRUCTION. EXPERIENCE OR TRAINING SHALL BE CONSIDERED TO BE RELEVANT WHERE THE DOCUMENTED EXPERIENCE OR TRAINING IS RELATED IN COMPLEXITY TO THE SAME TYPE OF SPECIAL INSPECTION OR TESTING ACTIVITIES FOR PROJECTS OF SIMILAR COMPLEXITY AND MATERIAL QUALITIES. THESE QUALIFICATIONS ARE IN ADDITION TO QUALIFICATIONS SPECIFIED IN OTHER SECTIONS OF THIS CODE. THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE AND ENGINEERS OF RECORD INVOLVED IN THE DESIGN OF THE PROJECT ARE PERMITTED TO ACT AS THE APPROVED AGENCY AND THEIR PERSONNEL ARE PERMITTED TO ACT AS SPECIAL INSPECTORS FOR THE WORK DESIGNED BY THEM, PROVIDED THEY QUALIFY AS SPECIAL INSPECTORS.

   B. ACCESS FOR SPECIAL INSPECTION: THE CONSTRUCTION OR WORK FOR WHICH SPECIAL INSPECTION OR TESTING IS REQUIRED SHALL REMAIN ACCESSIBLE AND EXPOSED FOR SPECIAL INSPECTION OR TESTING PURPOSES UNTIL COMPLETION OF THE REQUIRED SPECIAL INSPECTIONS OR TESTS.

   C. STATEMENT OF SPECIAL INSPECTIONS: THE APPLICANT SHALL SUBMIT A STATEMENT OF SPECIAL INSPECTIONS IN ACCORDANCE WITH SECTION 107.1 OF THE BUILDING CODE AS LISTED IN 1.0 AS A CONDITION FOR PERMIT ISSUANCE. THIS STATEMENT SHALL BE IN ACCORDANCE WITH SECTION 1704.3 OF THE BUILDING CODE AS LISTED IN 1.0.

   D. REPORT REQUIREMENT: APPROVED AGENCIES SHALL KEEP RECORDS OF SPECIAL INSPECTIONS AND TESTS. THE APPROVED AGENCY SHALL SUBMIT REPORTS OF SPECIAL INSPECTIONS AND TESTS TO THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. REPORTS SHALL INDICATE THAT WORK INSPECTED OR TESTED WAS OR WAS NOT COMPLETED IN CONFORMANCE TO APPROVED CONSTRUCTION DOCUMENTS. DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THEY ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE PRIOR TO THE COMPLETION OF THAT PHASE OF THE WORK. A FINAL REPORT DOCUMENTING REQUIRED SPECIAL INSPECTIONS AND TESTS, AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS OR TESTS, SHALL BE SUBMITTED AT A POINT IN TIME AGREED UPON PRIOR TO THE START OF WORK BY THE OWNER OR THE OWNER'S AUTHORIZED AGENT TO THE BUILDING OFFICIAL.

   E. SPECIAL INSPECTION OF FABRICATED ITEMS: WHERE FABRICATION OF STRUCTURAL, LOAD-BEARING OR LATERAL LOAD-RESISTING MEMBERS OR ASSEMBLIES IS BEING CONDUCTED ON THE PREMISES OF A FABRICATOR'S SHOP, SPECIAL INSPECTIONS OF THE FABRICATED ITEMS SHALL BE PERFORMED DURING FABRICATION, EXCEPT WHERE THE FABRICATOR HAS BEEN APPROVED TO PERFORM WORK WITHOUT SPECIAL INSPECTIONS IN ACCORDANCE WITH SECTION 1704.2.5.1 OF THE BUILDING CODE AS LISTED IN 1.0.

   F. FABRICATOR APPROVAL: SPECIAL INSPECTIONS DURING FABRICATION ARE NOT REQUIRED WHERE THE WORK IS DONE ON THE PREMISES OF A FABRICATOR APPROVED TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION. APPROVAL SHALL BE BASED ON REVIEW OF THE FABRICATOR'S WRITTEN FABRICATION PROCEDURES AND QUALITY CONTROL MANUALS THAT PROVIDE A BASIS FOR CONTROL OF MATERIALS AND WORKMANSHIP, WITH PERIODIC AUDITING OF FABRICATION AND QUALITY CONTROL PRACTICES BY AN APPROVED AGENCY OR THE BUILDING OFFICIAL. AT COMPLETION OF FABRICATION, THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE OWNER OR THE OWNER'S AUTHORIZED AGENT FOR SUBMITTAL TO THE BUILDING OFFICIAL AS SPECIFIED IN SECTION 1704.5 OF THE BUILDING CODE AS LISTED IN 1.0 STATING THAT THE WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.

3. STATEMENT OF SPECIAL INSPECTIONS: WHERE SPECIAL INSPECTIONS OR TESTS ARE REQUIRED BY SECTION 1705 OF THE BUILDING CODE AS LISTED IN 1.0, THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE SHALL PREPARE A STATEMENT OF SPECIAL INSPECTIONS IN ACCORDANCE WITH SECTION 1704.3.1 OF THE BUILDING CODE AS LISTED IN 1.0 FOR SUBMITTAL BY THE APPLICANT IN ACCORDANCE WITH SECTION 1704.2.3 OF THE BUILDING CODE AS LISTED IN 1.0.

   EXCEPTIONS: THE STATEMENT OF SPECIAL INSPECTIONS IS PERMITTED TO BE PREPARED BY A QUALIFIED PERSON APPROVED BY THE BUILDING OFFICIAL FOR CONSTRUCTION NOT DESIGNED BY A REGISTERED DESIGN PROFESSIONAL.

   A. CONTENT OF STATEMENT OF SPECIAL INSPECTIONS: THE STATEMENT OF SPECIAL INSPECTIONS SHALL IDENTIFY THE FOLLOWING:

   - THE MATERIALS, SYSTEMS, COMPONENTS AND WORK REQUIRED TO HAVE SPECIAL INSPECTIONS OR TESTS BY THE BUILDING OFFICIAL OR BY THE REGISTERED
   - DESIGN PROFESSIONAL RESPONSIBLE FOR EACH PORTION OF THE WORK.
   - THE TYPE AND EXTENT OF EACH SPECIAL INSPECTION.
   - ADDITIONAL REQUIREMENTS FOR SPECIAL INSPECTIONS OR TESTS FOR SEISMIC OR WIND RESISTANCE AS SPECIFIED IN SECTIONS 1705.11, 1705.12 AND 1705.13 OF THE BUILDING CODE AS LISTED IN 1.0.
   - FOR EACH TYPE OF SPECIAL INSPECTION, IDENTIFICATION AS TO WHETHER IT WILL BE CONTINUOUS SPECIAL INSPECTION, PERIODIC SPECIAL INSPECTION OR PERFORMED IN ACCORDANCE WITH THE NOTATION USED IN THE REFERENCED STANDARD WHERE THE INSPECTIONS ARE DEFINED

   B. SEISMIC REQUIREMENTS IN THE STATEMENT OF SPECIAL INSPECTIONS: WHERE SECTION 1705.12 OR 1705.13 OF THE BUILDING CODE AS LISTED IN 1.0 SPECIFIES SPECIAL INSPECTIONS OR TESTS FOR SEISMIC RESISTANCE, THE STATEMENT OF SPECIAL INSPECTIONS SHALL IDENTIFY THE DESIGNATED SEISMIC SYSTEMS AND SEISMIC FORCE-RESISTING SYSTEMS THAT ARE SUBJECT TO THE SPECIAL INSPECTIONS OR TESTS.

   C. WIND REQUIREMENTS IN THE STATEMENT OF SPECIAL INSPECTIONS: WHERE SECTION 1705.11 OF THE BUILDING CODE AS LISTED IN 1.0 SPECIFIES SPECIAL INSPECTION FOR WIND RESISTANCE, THE STATEMENT OF SPECIAL INSPECTIONS SHALL IDENTIFY THE MAIN WINDFORCE-RESISTING SYSTEMS AND WIND-RESISTING COMPONENTS THAT ARE SUBJECT TO SPECIAL INSPECTIONS.

4. CONTRACTOR RESPONSIBILITY: EACH CONTRACTOR RESPONSIBLE FOR THE CONSTRUCTION OF A MAIN WIND- OR SEISMIC FORCE-RESISTING SYSTEM, DESIGNATED SEISMIC SYSTEM OR A WIND- OR SEISMIC FORCE-RESISTING COMPONENT LISTED IN THE STATEMENT OF SPECIAL INSPECTIONS SHALL SUBMIT A WRITTEN STATEMENT OF RESPONSIBILITY TO THE BUILDING OFFICIAL AND THE OWNER OR THE OWNER'S AUTHORIZED AGENT PRIOR TO THE COMMENCEMENT OF WORK ON THE SYSTEM OR COMPONENT. THE CONTRACTOR'S STATEMENT OF RESPONSIBILITY SHALL CONTAIN ACKNOWLEDGEMENT OF AWARENESS OF THE SPECIAL REQUIREMENTS CONTAINED IN THE STATEMENT OF SPECIAL INSPECTIONS.

5. SUBMITTALS TO THE BUILDING OFFICIAL: IN ADDITION TO THE SUBMITTAL OF REPORTS OF SPECIAL INSPECTIONS AND TESTS IN ACCORDANCE WITH SECTION 1704.2.4 OF THE BUILDING CODE AS LISTED IN 1.0, REPORTS AND CERTIFICATES SHALL BE SUBMITTED BY THE OWNER OR THE OWNER'S AUTHORIZED AGENT TO THE BUILDING OFFICIAL FOR EACH OF THE FOLLOWING IN ACCORDANCE WITH SECTION 1704.5 OF THE BUILDING CODE AS LISTED IN 1.0.

6. STRUCTURAL OBSERVATIONS: WHERE REQUIRED BY THE PROVISIONS OF SECTION 1704.6.1, 1704.6.2 OR 1704.6.3 OF THE BUILDING CODE AS LISTED IN 1.0, THE OWNER OR THE OWNER'S AUTHORIZED AGENT SHALL EMPLOY A REGISTERED DESIGN PROFESSIONAL TO PERFORM STRUCTURAL OBSERVATIONS. STRUCTURAL OBSERVATION DOES NOT INCLUDE OR WAIVE THE RESPONSIBILITY FOR THE INSPECTIONS IN SECTION 110 OF THE BUILDING CODE AS LISTED IN 1.0 OR THE SPECIAL INSPECTIONS IN SECTION 1705 OR OTHER SECTIONS OF THIS CODE. PRIOR TO THE COMMENCEMENT OF OBSERVATIONS, THE STRUCTURAL OBSERVER SHALL SUBMIT TO THE BUILDING OFFICIAL A WRITTEN STATEMENT IDENTIFYING THE FREQUENCY AND EXTENT OF STRUCTURAL OBSERVATIONS. AT THE CONCLUSION OF THE WORK INCLUDED IN THE PERMIT, THE STRUCTURAL OBSERVER SHALL SUBMIT TO THE BUILDING OFFICIAL A WRITTEN STATEMENT THAT THE SITE VISITS HAVE BEEN MADE AND IDENTIFY ANY REPORTED DEFICIENCIES THAT, TO THE BEST OF THE STRUCTURAL OBSERVER'S KNOWLEDGE, HAVE NOT BEEN RESOLVED.

   A. STRUCTURAL OBSERVATIONS FOR STRUCTURES: STRUCTURAL OBSERVATIONS SHALL BE PROVIDED FOR THOSE STRUCTURES WHERE ONE OR MORE OF THE FOLLOWING CONDITIONS EXIST:

   - THE STRUCTURE IS CLASSIFIED AS RISK CATEGORY IV.
   - THE STRUCTURE IS A HIGH-RISE BUILDING.
   - SUCH OBSERVATION IS REQUIRED BY THE REGISTERED DESIGN PROFESSIONAL RESPONSIBLE FOR THE STRUCTURAL DESIGN.
   - SUCH OBSERVATION IS SPECIFICALLY REQUIRED BY THE BUILDING OFFICIAL.

   B. STRUCTURAL OBSERVATIONS FOR SEISMIC RESISTANCE: STRUCTURAL OBSERVATIONS SHALL BE PROVIDED FOR THOSE STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY D, E OR F WHERE ONE OR MORE OF THE FOLLOWING CONDITIONS EXIST:

   - THE STRUCTURE IS CLASSIFIED AS RISK CATEGORY III OR IV.
   - THE STRUCTURE IS ASSIGNED TO SEISMIC DESIGN CATEGORY E, IS CLASSIFIED AS RISK CATEGORY I OR II, AND IS GREATER THAN TWO STORIES ABOVE THE GRADE PLANE.

   C. STRUCTURAL OBSERVATIONS FOR WIND RESISTANCE: STRUCTURAL OBSERVATIONS SHALL BE PROVIDED FOR THOSE STRUCTURES SITED WHERE V IS 130 MPH (58 M/SEC) OR GREATER AND THE STRUCTURE IS CLASSIFIED AS RISK CATEGORY III OR IV.

REQUIRED SPECIAL INSPECTIONS AND TESTS

1. GENERAL: SPECIAL INSPECTIONS AND TESTS OF ELEMENTS AND NONSTRUCTURAL COMPONENTS OF BUILDINGS AND STRUCTURES SHALL MEET THE APPLICABLE REQUIREMENTS OF THIS SECTION.

   A. SPECIAL CASES: SPECIAL INSPECTIONS AND TESTS SHALL BE REQUIRED FOR PROPOSED WORK THAT IS, IN THE OPINION OF THE BUILDING OFFICIAL, UNUSUAL IN ITS NATURE, SUCH AS, BUT NOT LIMITED TO, THE FOLLOWING EXAMPLES:

   - CONSTRUCTION MATERIALS AND SYSTEMS THAT ARE ALTERNATIVES TO MATERIALS AND SYSTEMS PRESCRIBED BY THIS CODE.
   - UNUSUAL DESIGN APPLICATIONS OF MATERIALS DESCRIBED IN THIS CODE.
   - MATERIALS AND SYSTEMS REQUIRED TO BE INSTALLED IN ACCORDANCE WITH ADDITIONAL MANUFACTURER'S INSTRUCTIONS THAT PRESCRIBE REQUIREMENTS NOT CONTAINED IN THIS CODE OR IN STANDARDS REFERENCED BY THIS CODE.

2. STEEL CONSTRUCTION: THE SPECIAL INSPECTIONS AND NONDESTRUCTIVE TESTING OF STEEL CONSTRUCTION IN BUILDINGS, STRUCTURES, AND PORTIONS THEREOF SHALL BE IN ACCORDANCE WITH THIS SECTION.

   EXCEPTIONS: SPECIAL INSPECTIONS OF THE STEEL FABRICATION PROCESS SHALL NOT BE REQUIRED WHERE THE FABRICATION PROCESS FOR THE ENTIRE BUILDING OR STRUCTURE DOES NOT INCLUDE ANY WELDING, THERMAL CUTTING OR HEATING OPERATION OF ANY KIND. IN SUCH CASES, THE FABRICATOR SHALL BE REQUIRED TO SUBMIT A DETAILED PROCEDURE FOR MATERIAL CONTROL THAT DEMONSTRATES THE FABRICATOR'S ABILITY TO MAINTAIN SUITABLE RECORDS AND PROCEDURES SUCH THAT, AT ANY TIME DURING THE FABRICATION PROCESS, THE MATERIAL SPECIFICATION AND GRADE FOR THE MAIN STRESS-CARRYING ELEMENTS ARE CAPABLE OF BEING DETERMINED. MILL TEST REPORTS SHALL BE IDENTIFIABLE TO THE MAIN STRESS-CARRYING ELEMENTS WHERE REQUIRED BY THE APPROVED CONSTRUCTION DOCUMENTS.

   A. STRUCTURAL STEEL: SPECIAL INSPECTIONS AND NONDESTRUCTIVE TESTING OF STRUCTURAL STEEL ELEMENTS IN BUILDINGS, STRUCTURES AND PORTIONS THEREOF SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE INSPECTION REQUIREMENTS OF AISC 360.

   EXCEPTIONS: SPECIAL INSPECTION OF RAILING SYSTEMS COMPOSED OF STRUCTURAL STEEL ELEMENTS SHALL BE LIMITED TO WELDING INSPECTION OF WELDS AT THE BASE OF CANTILEVERED RAIL POSTS.

3. CONCRETE CONSTRUCTION: SPECIAL INSPECTIONS AND TESTS OF CONCRETE CONSTRUCTION SHALL BE PERFORMED IN ACCORDANCE WITH THIS SECTION AND TABLE 1705.3 OF THE BUILDING CODE AS LISTED IN 1.0.

   EXCEPTIONS: SPECIAL INSPECTIONS AND TESTS SHALL NOT BE REQUIRED FOR:

   - ISOLATED SPREAD CONCRETE FOOTINGS OF BUILDINGS THREE STORIES OR LESS ABOVE GRADE PLANE THAT ARE FULLY SUPPORTED ON EARTH OR ROCK.
   - CONTINUOUS CONCRETE FOOTINGS SUPPORTING WALLS OF BUILDINGS THREE STORIES OR LESS ABOVE GRADE PLANE THAT ARE FULLY SUPPORTED ON EARTH OR ROCK WHERE:
     - THE FOOTINGS SUPPORT WALLS OF LIGHT-FRAME CONSTRUCTION.
     - THE FOOTINGS ARE DESIGNED IN ACCORDANCE WITH TABLE 1809.7 OF THE BUILDING CODE AS LISTED IN 1.0.
     - THE STRUCTURAL DESIGN OF THE FOOTING IS BASED ON A SPECIFIED COMPRESSIVE STRENGTH, f'c, NOT MORE THAN 2,500 POUNDS PER SQUARE INCH (PSI) (17.2 MPA), REGARDLESS OF THE COMPRESSIVE STRENGTH SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS OR USED IN THE FOOTING CONSTRUCTION.
     - NONSTRUCTURAL CONCRETE SLABS SUPPORTED DIRECTLY ON THE GROUND, INCLUDING PRESTRESSED SLABS ON GRADE, WHERE THE EFFECTIVE PRESTRESS IN THE CONCRETE IS LESS THAN 150 PSI (1.03 MPA).
     - CONCRETE FOUNDATION WALLS CONSTRUCTED IN ACCORDANCE WITH TABLE 1807.1.6.2 OF THE BUILDING CODE AS LISTED IN 1.0.
     - CONCRETE PATIOS, DRIVEWAYS AND SIDEWALKS, ON GRADE.

   A. WELDING OF REINFORCIING BARS: SPECIAL INSPECTIONS OF WELDING AND QUALIFICATIONS OF SPECIAL INSPECTORS FOR REINFORCING BARS SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF AWS D1.4 FOR SPECIAL INSPECTION AND OF AWS D1.4 FOR SPECIAL INSPECTOR QUALIFICATION.

   B. MATERIAL TESTS: IN THE ABSENCE OF SUFFICIENT DATA OR DOCUMENTATION PROVIDING EVIDENCE OF CONFORMANCE TO QUALITY STANDARDS FOR MATERIALS IN CHAPTERS 19 AND 20 OF ACI 318, THE BUILDING OFFICIAL SHALL REQUIRE TESTING OF MATERIALS IN ACCORDANCE WITH THE APPROPRIATE STANDARDS AND CRITERIA FOR THE MATERIAL IN CHAPTERS 19 AND 20 OF ACI 318.

5. WOOD CONSTRUCTION: SPECIAL INSPECTIONS OF PREFABRICATED WOOD STRUCTURAL ELEMENTS AND ASSEMBLIES SHALL BE IN ACCORDANCE WITH SECTION 1704.2.5 OF THE BUILDING CODE AS LISTED IN 1.0. SPECIAL INSPECTIONS OF SITE-BUILT ASSEMBLIES SHALL BE IN ACCORDANCE WITH THIS SECTION.

   A. HIGH-LOAD DIAPHRAGMS: HIGH-LOAD DIAPHRAGMS DESIGNED IN ACCORDANCE WITH SECTION 2306.2 SHALL BE INSTALLED WITH SPECIAL INSPECTIONS AS INDICATED IN SECTION 1704.2 OF THE BUILDING CODE AS LISTED IN 1.0. THE SPECIAL INSPECTOR SHALL INSPECT THE WOOD STRUCTURAL PANEL SHEATHING TO ASCERTAIN WHETHER IT IS OF THE GRADE AND THICKNESS SHOWN ON THE APPROVED CONSTRUCTION DOCUMENTS. ADDITIONALLY, THE SPECIAL INSPECTOR MUST VERIFY THE NOMINAL SIZE OF FRAMING MEMBERS AT ADJOINING PANEL EDGES, THE NAIL OR STAPLE DIAMETER AND LENGTH, THE NUMBER OF FASTENER LINES AND THAT THE SPACING BETWEEN FASTENERS IN EACH LINE AND AT EDGE MARGINS AGREES WITH THE APPROVED CONSTRUCTION DOCUMENTS.

   B. METAL-PLATE-CONNECTED WOOD TRUSSES: SPECIAL INSPECTIONS OF WOOD TRUSSES WITH OVERALL HEIGHTS OF 60 INCHES (1524 MM) OR GREATER SHALL BE PERFORMED TO VERIFY THAT THE INSTALLATION OF THE PERMANENT INDIVIDUAL TRUSS MEMBER RESTRAINT/BRACING HAS BEEN INSTALLED IN ACCORDANCE WITH THE APPROVED TRUSS SUBMITTAL PACKAGE. FOR WOOD TRUSSES WITH A CLEAR SPAN OF 60 FEET (18288 MM) OR GREATER, THE SPECIAL INSPECTOR SHALL VERIFY DURING CONSTRUCTION THAT THE TEMPORARY INSTALLATION RESTRAINT/BRACING IS INSTALLED IN ACCORDANCE WITH THE APPROVED TRUSS SUBMITTAL PACKAGE.

6. SOILS: SPECIAL INSPECTIONS AND TESTS OF EXISTING SITE SOIL CONDITIONS, FILL PLACEMENT AND LOAD-BEARING REQUIREMENTS SHALL BE PERFORMED IN ACCORDANCE WITH THIS SECTION AND TABLE 1705.6 OF THE BUILDING CODE AS LISTED IN 1.0. THE APPROVED GEOTECHNICAL REPORT AND THE CONSTRUCTION DOCUMENTS PREPARED BY THE REGISTERED DESIGN PROFESSIONALS SHALL BE USED TO DETERMINE COMPLIANCE. DURING FILL PLACEMENT, THE SPECIAL INSPECTOR SHALL VERIFY THAT PROPER MATERIALS AND PROCEDURES ARE USED IN ACCORDANCE WITH THE PROVISIONS OF THE APPROVED GEOTECHNICAL REPORT.

   EXCEPTIONS: WHERE SECTION 1803 OF THE BUILDING CODE AS LISTED IN 1.0 DOES NOT REQUIRE REPORTING OF MATERIALS AND PROCEDURES FOR FILL PLACEMENT, THE SPECIAL INSPECTOR SHALL VERIFY THAT THE IN-PLACE DRY DENSITY OF THE COMPACTED FILL IS NOT LESS THAN 90 PERCENT OF THE MAXIMUM DRY DENSITY AT OPTIMUM MOISTURE CONTENT DETERMINED IN ACCORDANCE WITH ASTM D1557.

7. FABRICATED ITEMS: SPECIAL INSPECTIONS OF FABRICATED ITEMS SHALL BE PERFORMED IN ACCORDANCE WITH SECTION 1704.2.5 OF THE BUILDING CODE AS LISTED IN 1.0.

8. SPECIAL INSPECTIONS FOR WIND RESISTANCE: SPECIAL INSPECTIONS FOR WIND RESISTANCE SPECIFIED IN SECTIONS 1705.11.1 THROUGH 1705.11.3 OF THE BUILDING CODE AS LISTED IN 1.0, UNLESS EXEMPTED BY THE EXCEPTIONS TO SECTION 1704.2, ARE REQUIRED FOR BUILDINGS AND STRUCTURES CONSTRUCTED IN THE FOLLOWING AREAS:

   - IN WIND EXPOSURE CATEGORY B, WHERE VASD AS DETERMINED IN ACCORDANCE WITH SECTION 1609.3.1 OF THE BUILDING CODE AS LISTED IN 1.0 IS 120 MILES PER HOUR (52.8 M/S) OR GREATER.
   - IN WIND EXPOSURE CATEGORY C OR D, WHERE VASD AS DETERMINED IN ACCORDANCE WITH SECTION 1609.3.1 OF THE BUILDING CODE AS LISTED IN 1.0 IS 110 MPH (49 M/S) OR GREATER.

   A. STRUCTURAL WOOD: CONTINUOUS SPECIAL INSPECTION IS REQUIRED DURING FIELD GLUING OPERATIONS OF ELEMENTS OF THE MAIN WINDFORCE-RESISTING SYSTEM. PERIODIC SPECIAL INSPECTION IS REQUIRED FOR NAILING, BOLTING, ANCHORING AND OTHER FASTENING OF ELEMENTS OF THE MAIN WINDFORCE-RESISTING SYSTEM, INCLUDING WOOD SHEAR WALLS, WOOD DIAPHRAGMS, DRAG STRUTS, BRACES AND HOLDOWNS.

   EXCEPTIONS: SPECIAL INSPECTIONS ARE NOT REQUIRED FOR WOOD SHEAR WALLS, SHEAR PANELS AND DIAPHRAGMS, INCLUDING NAILING, BOLTING, ANCHORING AND OTHER FASTENING TO OTHER ELEMENTS OF THE MAIN WINDFORCE-RESISTING SYSTEM, WHERE THE SPECIFIED FASTENER SPACING AT PANEL EDGES IS MORE THAN 4 INCHES (102 MM) ON CENTER.

   B. WIND-RESISTING COMPONENTS: PERIODIC SPECIAL INSPECTION IS REQUIRED FOR FASTENING OF THE FOLLOWING SYSTEMS AND COMPONENTS:

   - ROOF COVERING, ROOF DECK AND ROOF FRAMING CONNECTIONS.
   - EXTERIOR WALL COVERING AND WALL CONNECTIONS TO ROOF AND FLOOR DIAPHRAGMS AND FRAMING.

9. SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE: SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE SHALL BE REQUIRED AS SPECIFIED IN SECTIONS 1705.12.1 THROUGH 1705.12.9 OF THE BUILDING CODE AS LISTED IN 1.0, UNLESS EXEMPTED BY THE EXCEPTIONS OF SECTION 1704.2 OF THE BUILDING CODE AS LISTED IN 1.0.

   EXCEPTIONS: THE SPECIAL INSPECTIONS SPECIFIED IN SECTIONS 1705.12.1 THROUGH 1705.12.9 OF THE BUILDING CODE AS LISTED IN 1.0 ARE NOT REQUIRED FOR STRUCTURES DESIGNED AND CONSTRUCTED IN ACCORDANCE WITH ONE OF THE FOLLOWING:

   - THE STRUCTURE CONSISTS OF LIGHT-FRAME CONSTRUCTION: THE DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, SDS, AS DETERMINED IN SECTION 1613.2.4 OF THE BUILDING CODE AS LISTED IN 1.0, DOES NOT EXCEED 0.5; AND THE BUILDING HEIGHT OF THE STRUCTURE DOES NOT EXCEED 35 FEET (10668 MM).
   - THE SEISMIC FORCE-RESISTING SYSTEM OF THE STRUCTURE CONSISTS OF REINFORCED MASONRY OR REINFORCED CONCRETE; THE DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, SDS, AS DETERMINED IN SECTION 1613.2.4, DOES NOT EXCEED 0.5; AND THE BUILDING HEIGHT OF THE STRUCTURE DOES NOT EXCEED 25 FEET (7620 MM).
   - THE STRUCTURE IS A DETACHED ONE- OR TWO-FAMILY DWELLING NOT EXCEEDING TWO STORIES ABOVE GRADE PLANE AND DOES NOT HAVE ANY OF THE FOLLOWING HORIZONTAL OR VERTICAL IRREGULARITIES IN ACCORDANCE WITH SECTION 12.3 OF ASCE 7:

   -TORSIONAL OR EXTREME TORSIONAL IRREGULARITY.
   -NON-PARALLEL SYSTEMS IRREGULARITY.
   -STIFFNESS-SOFT STORY OR STIFFNESS-EXTREME SOFT STORY IRREGULARITY.
   -DISCONTINUITY IN LATERAL STRENGTH-WEAK STORY IRREGULARITY.

   A. STRUCTURAL STEEL: SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE SHALL BE IN ACCORDANCE WITH SECTION 1705.12.1.1 OR 1705.12.1.2 OF THE BUILDING CODE AS LISTED IN 1.0, AS APPLICABLE.

   A1. SEISMIC FORCE-RESISTING SYSTEMS: SPECIAL INSPECTIONS OF STRUCTURAL STEEL IN THE SEISMIC FORCE-RESISTING SYSTEMS IN BUILDINGS AND STRUCTURES ASSIGNED TOSEISMIC DESIGN CATEGORY B, C, D, E OR F SHALL BE PERFORMED IN ACCORDANCE WITH THE QUALITY ASSURANCE REQUIREMENTS OF AISC 341.

   EXCEPTIONS:

   -IN BUILDINGS AND STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY B OR C, SPECIAL INSPECTIONS ARE NOT REQUIRED FOR STRUCTURAL STEEL SEISMIC FORCE-RESISTING SYSTEMS WHERE THE RESPONSE MODIFICATION COEFFICIENT, R, DESIGNATED FOR "STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE, EXCLUDING CANTILEVER COLUMN SYSTEMS" IN ASCE 7, TABLE 12.2-1, HAS BEEN USED FOR DESIGN AND DETAILING.

   -IN STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY D, E, OR F, SPECIAL INSPECTIONS ARE NOT REQUIRED FOR STRUCTURAL STEEL SEISMIC FORCE-RESISTING SYSTEMS WHERE DESIGN AND DETAILING IN ACCORDANCE WITH AISC 360 IS PERMITTED BY ASCE 7, TABLE 15.4-1.

   A2. STRUCTURAL STEEL ELEMENTS: SPECIAL INSPECTIONS OF STRUCTURAL STEEL ELEMENTS IN THE SEISMIC FORCE-RESISTING SYSTEMS OF BUILDINGS AND STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY B, C, D, E OR F OTHER THAN THOSE COVERED IN SECTION 1705.12.1.1, INCLUDING STRUTS, COLLECTORS, CHORDS AND FOUNDATION ELEMENTS, SHALL BE PERFORMED IN ACCORDANCE WITH THE QUALITY ASSURANCE REQUIREMENTS OF AISC 341.

   EXCEPTIONS:

   -IN BUILDINGS AND STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY B OR C, SPECIAL INSPECTIONS OF STRUCTURAL STEEL ELEMENTS ARE NOT REQUIRED FOR SEISMIC FORCE-RESISTING SYSTEMS WITH A RESPONSE MODIFICATION COEFFICIENT, R, OF 3 OR LESS.

   -IN STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY D, E, OR F, SPECIAL INSPECTIONS OF STRUCTURAL STEEL ELEMENTS ARE NOT REQUIRED FOR SEISMIC FORCE-RESISTING SYSTEMS WHERE DESIGN AND DETAILING OTHER THAN AISC 341 IS PERMITTED BY ASCE 7, TABLE 15.4-1. SPECIAL INSPECTION SHALL BE IN ACCORDANCE WITH THE APPLICABLE REFERENCED STANDARD LISTED IN ASCE 7, TABLE 15.4-1.

   B. STRUCTURAL WOOD: FOR THE SEISMIC FORCE-RESISTING SYSTEMS OF STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY C, D, E OR F:

   - CONTINUOUS SPECIAL INSPECTION SHALL BE REQUIRED DURING FIELD GLUING OPERATIONS OF ELEMENTS OF THE SEISMIC FORCE-RESISTING SYSTEM.
   - PERIODIC SPECIAL INSPECTION SHALL BE REQUIRED FOR NAILING, BOLTING, ANCHORING AND OTHER FASTENING OF ELEMENTS OF THE SEISMIC FORCE-RESISTING SYSTEM, INCLUDING WOOD SHEAR WALLS, WOOD DIAPHRAGMS, DRAG STRUTS, BRACES, SHEAR PANELS AND HOLDOWNS.

   EXCEPTIONS: SPECIAL INSPECTIONS ARE NOT REQUIRED FOR WOOD SHEAR WALLS, SHEAR PANELS AND DIAPHRAGMS, INCLUDING NAILING, BOLTING, ANCHORING AND OTHER FASTENING TO OTHER ELEMENTS OF THE SEISMIC FORCE-RESISTING SYSTEM, WHERE THE FASTENER SPACING OF THE SHEATHING IS MORE THAN 4 INCHES (102 MM) ON CENTER.

   C. COLD-FORMED STEEL LIGHT-FRAME CONSTRUCTION: FOR THE SEISMIC FORCE-RESISTING SYSTEMS OF STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY C, D, E OR F, PERIODIC SPECIAL INSPECTION SHALL BE REQUIRED FOR BOTH:

   - WELDING OPERATIONS OF ELEMENTS OF THE SEISMIC FORCE-RESISTING SYSTEM.
   - SCREW ATTACHMENT, BOLTING, ANCHORING AND OTHER FASTENING OF ELEMENTS OF THE SEISMIC FORCE-RESISTING SYSTEM, INCLUDING SHEAR WALLS, BRACES, DIAPHRAGMS, COLLECTORS (DRAG STRUTS) AND HOLDOWNS.

   EXCEPTIONS: SPECIAL INSPECTIONS ARE NOT REQUIRED FOR COLD-FORMED STEEL LIGHT-FRAME SHEAR WALLS AND DIAPHRAGMS, INCLUDING SCREW INSTALLATION, BOLTING, ANCHORING AND OTHER FASTENING TO COMPONENTS OF THE SEISMIC FORCE-RESISTING SYSTEM, WHERE EITHER OF THE FOLLOWING APPLIES:

   - THE SHEATHING IS GYPSUM BOARD OR FIBER BOARD.
   - THE SHEATHING IS WOOD STRUCTURAL PANEL OR STEEL SHEETS ON ONLY ONE SIDE OF THE SHEAR WALL, SHEAR PANEL OR DIAPHRAGM ASSEMBLY AND THE FASTENER SPACING OF THE SHEATHING IS MORE THAN 4 INCHES (102 MM) ON CENTER.

   D. DESIGNATED SEISMIC SYSTEMS: FOR STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY C, D, E OR F, THE SPECIAL INSPECTOR SHALL EXAMINE DESIGNATED SEISMIC SYSTEMS REQUIRING SEISMIC QUALIFICATION IN ACCORDANCE WITH SECTION 13.2.2 OF ASCE 7 AND VERIFY THAT THE LABEL, ANCHORAGE AND MOUNTING CONFORM TO THE CERTIFICATE OF COMPLIANCE.

10. TESTING FOR SEISMIC RESISTANCE: TESTING FOR SEISMIC RESISTANCE SHALL BE REQUIRED AS SPECIFIED IN SECTIONS 1705.13.1 THROUGH 1705.13.4 OF THE BUILDING CODE AS LISTED IN 1.0, UNLESS EXEMPTED FROM SPECIAL INSPECTIONS BY THE EXCEPTIONS OF SECTION 1704.2 OF THE BUILDING CODE AS LISTED IN 1.0.

   A. STRUCTURAL STEEL: NONDESTRUCTIVE TESTING FOR SEISMIC RESISTANCE SHALL BE IN ACCORDANCE WITH SECTION 1705.13.1.1 OR 1705.13.1.2 OF THE BUILDING CODE AS LISTED IN 1.0, AS APPLICABLE.

   A1. SEISMIC FORCE-RESISTING SYSTEMS: NONDESTRUCTIVE TESTING OF STRUCTURAL STEEL IN THE SEISMIC FORCE-RESISTING SYSTEMS IN BUILDINGS AND STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY B, C, D, E OR F SHALL BE PERFORMED IN ACCORDANCE WITH THE QUALITY ASSURANCE REQUIREMENTS OF AISC 341.

   EXCEPTIONS:

   - IN BUILDINGS AND STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY B OR C, NONDESTRUCTIVE TESTING IS NOT REQUIRED FOR STRUCTURAL STEEL SEISMIC FORCE-RESISTING SYSTEMS WHERE THE RESPONSE MODIFICATION COEFFICIENT, R, DESIGNATED FOR "STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE, EXCLUDING CANTILEVER COLUMN SYSTEMS" IN ASCE 7, TABLE 12.2-1, HAS BEEN USED FOR DESIGN AND DETAILING.
   - IN STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY D, E, OR F, NONDESTRUCTIVE TESTING IS NOT REQUIRED FOR STRUCTURAL STEEL SEISMIC FORCE-RESISTING SYSTEMS WHERE DESIGN AND DETAILING IN ACCORDANCE WITH AISC 360 IS PERMITTED BY ASCE 7, TABLE 15.4-1.

   A2. STRUCTURAL STEEL ELEMENTS: NONDESTRUCTIVE TESTING OF STRUCTURAL STEEL ELEMENTS IN THE SEISMIC FORCE-RESISTING SYSTEMS OF BUILDINGS AND STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY B, C, D, E OR F OTHER THAN THOSE COVERED IN SECTION 1705.13.1.1 OF THE BUILDING CODE AS LISTED IN 1.0., INCLUDING STRUTS, COLLECTORS, CHORDS AND FOUNDATION ELEMENTS, SHALL BE PERFORMED IN ACCORDANCE WITH THE QUALITY ASSURANCE REQUIREMENTS OF AISC 341.

   EXCEPTIONS:

   - IN BUILDINGS AND STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY B OR C, NONDESTRUCTIVE TESTING OF STRUCTURAL STEEL ELEMENTS IS NOT REQUIRED FOR SEISMIC FORCE-RESISTING SYSTEMS WITH A RESPONSE MODIFICATION COEFFICIENT, R, OF 3 OR LESS.
   - IN STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY D, E, OR F, NONDESTRUCTIVE TESTING IS NOT REQUIRED FOR STRUCTURAL STEEL SEISMIC FORCE-RESISTING SYSTEMS WHERE DESIGN AND DETAILING IN ACCORDANCE WITH AISC 360 IS PERMITTED BY ASCE 7, TABLE 15.4-1.

   B. DESIGNATED SEISMIC SYSTEMS: FOR STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY C, D, E OR F AND WITH DESIGNATED SEISMIC SYSTEMS THAT ARE SUBJECT TO THE REQUIREMENTS OF SECTION 13.2.2 OF ASCE 7 FOR CERTIFICATION, THE REGISTERED DESIGN PROFESSIONAL SHALL SPECIFY ON THE APPROVED CONSTRUCTION DOCUMENTS THE REQUIREMENTS TO BE MET BY ANALYSIS, TESTING OR EXPERIENCE DATA AS SPECIFIED THEREIN. CERTIFICATES OF COMPLIANCE DOCUMENTING THAT THE REQUIREMENTS ARE MET SHALL BE SUBMITTED TO THE BUILDING OFFICIAL AS SPECIFIED IN SECTION 1704.5 OF THE BUILDING CODE AS LISTED IN 1.0.

   C. SEISMIC ISOLATION SYSTEMS: SEISMIC ISOLATION SYSTEMS IN SEISMICALLY ISOLATED STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY B, C, D, E OR F SHALL BE TESTED IN ACCORDANCE WITH SECTION 17.8 OF ASCE 7.

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TROPICAL SMOOTHIE CAFE

SAGAMORE PARKWAY/CUMBERLAND AVENUE, LOT 13
WEST LAFAYETTE, IN 47906

REV	REV DESCRIPTION	DATE
	ISSUED FOR PERMIT	04/27/26

KETAN J PATEL
REGISTERED
No. PE11800241
STATE OF INDIANA
PROFESSIONAL ENGINEER
EXP: 07/31/2026
DATE: 06/17/2026

DESIGN PROFESSIONAL SEAL

Drawn / Designed by:	NA/KV
Reviewed by:	RU
Approved by:	KP
Project Number:	**KPJ-25-0282**

SHEET NAME:

SPECIAL INSPECTION NOTES

SHEET NUMBER:

S-103

SCHEDULE OF SPECIAL INSPECTIONS SERVICES

PROJECT		
MATERIAL / ACTIVITY	**SERVICE**	**EXTENT**
1705.1.1 Special Cases (work unusual in nature, including but not limited to alternative materials and systems, unusual design applications, materials and systems with special manufacturer's requirements - add additional rows as needed.)	Submittal review, shop (3) and/or field inspection	
1. Inspection of anchors post-installed in solid grouted masonry: Per research reports including verification of anchor type, anchor dimensions, hole dimensions, hole cleaning procedures, anchor spacing, edge distances, masonry unit, grout, masonry compressive strength, anchor embedment and tightening torque	Field inspection	Periodic or as required by the research report issued by an approved source
2. Aggregate Pier Inspection: The special inspector's responsibilities include, but are not limited to, review of the aggregate pier designer's use of soil parameters as presented in the project soils report, and during construction, verification of aggregate properties, type and number of lifts of aggregate, hole size and depths and top elevations of the pier elements, and applied energy. Additionally, results of qualitative tests on production aggregate pier elements such as modulus load testing, uplift pull-out testing, bottom stabilization tests and dynamic cone penetration tests, shall be reviewed to verify compliance with design specifications.	Field inspection	Periodic or as required by the research report issued by an approved source

1705.2.1 Structural Steel Construction

MATERIAL / ACTIVITY	SERVICE	EXTENT
1. Fabricator and erector documents (Verify reports and certificates as listed in AISC 360, Section N 3.2 for compliance with construction documents)	Submittal Review	Each submittal
2. Material verification of structural steel	Shop (3) and field inspection	Periodic
3. Structural steel welding:		
a. Inspection tasks Prior to Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-1)	Shop (3) and field inspection	Observe or Perform as noted (4)
b. Inspection tasks During Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-2)	Shop (3) and field inspection	Observe (4)
c. Inspection tasks After Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-3)	Shop (3) and field inspection	Observe or Perform as noted (4)
d. Nondestructive testing (NDT) of welded joints: *see Commentary*		
1) Complete penetration groove welds 5/16" or greater in *risk category* III or IV	Shop (3) or field ultrasonic testing - 100%	Periodic
2) Complete penetration groove welds 5/16" or greater in *risk category* II	Shop (3) or field ultrasonic testing - 10% of welds minimum	Periodic
3) Welded joints subject to fatigue when required by AISC 360, Appendix 3, Table A-3.1	Shop (3) or field radiographic or Ultrasonic testing	Periodic
4) Fabricator's NDT reports when fabricator performs NDT	Verify reports	Each submittal (5)
4. Structural steel bolting:	Shop (3) and field inspection	
a. Inspection tasks Prior to Bolting (Observe, or perform tasks for each bolted connection, in accordance with QA tasks listed in AISC 360, Table N5.6-1)		Observe or Perform as noted (4)
b. Inspection tasks During Bolting (Observe the QA tasks listed in AISC 360, Table N5.6-2)		Observe (4)
1) Pre-tensioned and slip-critical joints		
a) Turn-of-nut with matching markings		Periodic
b) Direct tension indicator		Periodic
c) Twist-off type tension control bolt		Periodic
d) Turn-of-nut without matching markings		Continuous
e) Calibrated wrench		Continuous
2) Snug-tight joints		Periodic
c. Inspection tasks After Bolting (Perform tasks for each bolted connection in accordance with QA tasks listed in AISC 360, Table N5.6-3)		Perform (4)
5. Visual inspection of exposed cut surfaces of galvanized structural steel main members and exposed corners of the rectangular HSS for cracks subsequent to galvanizing	Shop (3) or field inspection	Periodic
6. Embedments (Verify diameter, grade, type, length, embedment. See 1705.3 for anchors)	Field inspection	Periodic

1705.3 Concrete Construction

MATERIAL / ACTIVITY	SERVICE	EXTENT	REFERENCE STANDARD	IBC REFERENCE
1. Inspection and placement verification of reinforcing steel and prestressing tendons.	Shop (3) and field inspection	Periodic	ACI 318: Ch. 20, 25.2, 25.3, 26.6.1-26.6.3	-
2. Reinforcing bar welding:				
a. Verification of weldability of bars other than ASTM A706.		Periodic	AWS D1.4 ACI 318: 26.6.4	-
b. Inspection of single-pass fillet welds 5/16 or less in size.		Periodic		
c. Inspection of all other welds.		Continuous		
3. Inspection of anchors cast in concrete.	Shop (3) and field inspection	Periodic	ACI 318: 17.8.2	-
4. Inspection of anchors post-installed in hardened concrete members per research reports, or, if no specific requirements are provided, requirements shall be provided by the registered design professional and approved by the building official, including verification of anchor type, anchor dimensions, hole dimensions, hole cleaning procedures, anchor spacing, edge distances, concrete minimum thickness, anchor embedment and tightening torque	Field inspection	Periodic or as required by the research report issued by an approved source	ACI 318: 17.8.2.4 ACI 318: 17.8.2	-
a. Adhesive anchors installed in horizontal or upward-inclined orientation that resist sustained tension loads.		Continuous		
b. Mechanical and adhesive anchors note defined in 4a.		Periodic		
5. Verify use of approved design mix	Shop (3) and field inspection	Periodic	ACI 318: Ch. 19, 26.4.3, 26.4.4	1904.1,1904.2
6. Prior to placement, fresh concrete sampling, perform slump and air content tests and determine temperature of concrete and perform any other tests as specified in construction documents.	Shop (3) and field inspection	Continuous	ASTM C172 ASTM C31 ACI 318: 26.5, 26.12	1908.10
7. Inspection of concrete and shotcrete placement for proper application techniques	Shop (3) and field inspection	Continuous	ACI 318: 26.5	1908.6, 1908.7, 1908.8
8. Verify maintenance of specified curing temperature and techniques	Shop (3) and field inspection	Periodic	ACI 318: 26.5.3-26.5.5	1908.9
9. Inspection of prestressed concrete:	Shop (3) and field inspection		ACI 318: 26.10	-
a. Application of prestressing force		Continuous		
b. Grouting of bonded prestressing tendons		Continuous		
10. Inspect erection of precast concrete members		Periodic	ACI 318: 26.9	-
11. Verification of in-situ concrete strength, prior to stressing of tendons in post tensioned concrete and prior to removal of shores and forms from beams and structural slabs	Review field testing and laboratory reports	Periodic	ACI 318: 26.13.1.3 ACI 550.5	-
12. Inspection of formwork for shape, lines, location and dimensions	Field inspection	Periodic	ACI 318: 26.11.2	-
13. Concrete strength testing and verification of compliance with construction documents	Field testing and review of laboratory reports	Periodic	ACI 318: 26.11.1.2(b)	-

1705.5 Wood Construction

MATERIAL / ACTIVITY	SERVICE	EXTENT
1. For prefabricated wood structural elements, inspection of the fabrication process and assemblies in accordance with Section 1704.2.5.	In-plant review (3)	Periodic
2. For high-load diaphragms, verify grade and thickness of structural panel sheathing agree with approved building plans.	Field inspection	Periodic
3. For high-load diaphragms, verify nominal size of framing members at adjoining panel edges, nail or staple diameter and length, number of fastener lines, and that spacing between fasteners in each line and at edge margins agree with approved building plans	Field inspection	Periodic
4. Metal-plate-connected wood trusses:		
a. Verification that permanent individual truss member restraint/bracing has been installed in accordance with the approved truss submittal package when the truss height is greater than or equal to 60".	Field inspection	Periodic
b. For trusses spanning 60 feet or greater: verify temporary and permanent restraint/bracing are installed in accordance with the approved truss submittal package	Field inspection	Periodic

1705.6 Soils

MATERIAL / ACTIVITY	SERVICE	EXTENT
1. Verify materials below shallow foundations are adequate to achieve the design bearing capacity.	Field inspection	Periodic
2. Verify excavations are extended to proper depth and have reached proper material.	Field inspection	Periodic
3. Perform classification and testing of compacted fill materials.	Field inspection	Periodic
4. Verify use of proper materials, densities, and lift thicknesses during placement and compaction of compacted fill.	Field inspection	Continuous
5. Prior to placement of compacted fill, inspect subgrade and verify that site has been prepared properly.	Field inspection	Periodic

1705.11.1 Structural Wood Special Inspections For Wind Resistance

MATERIAL / ACTIVITY	SERVICE	EXTENT
1. Inspection of field gluing operations of elements of the main windforce-resisting system	Field inspection	Continuous
2. Inspection of nailing, bolting, anchoring and other fastening of components within the main windforce-resisting system, including wood shear walls, wood diaphragms, drag struts, braces and hold-downs.	Shop (3) and field inspection	Periodic

1705.12.2 Structural Wood Special Inspections for Seismic

MATERIAL / ACTIVITY	SERVICE	EXTENT
1. Field gluing operations of elements of the seismic-force resisting system for SDC C, D, E or F.	Field inspection	Continuous
2. Nailing, bolting, anchoring and other fastening of components within the seismic-force-resisting system including wood shear walls, wood diaphragms, drag struts, shear panels and hold-downs for SDC C, D, E or F.	Shop (3) and field inspection	Periodic

1705.13 Structural Steel Special Inspections for Seismic Resistance

MATERIAL / ACTIVITY	SERVICE	EXTENT
1. Seismic force-resisting systems in SDC B, C, D, E, or F.	Shop (3) and field inspection	In accordance with AISC 341
2. Structural steel elements in SDC B, C, D, E, or F other than those in Item 1. including struts, collectors, chords and foundation elements.	Shop (3) and field inspection	In accordance with AISC 341

1705.13.1 Structural Steel Testing for Seismic Resistance

MATERIAL / ACTIVITY	SERVICE	EXTENT
1. Nondestructive testing of structural steel in the seismic force-resisting systems in accordance with AISC 341 in structures assigned to SDC B, C, D, E or F.	Field test	Periodic
2. Nondestructive testing of structural steel elements in the seismic force-resisting systems not covered in 1 above including struts, collectors, chords and foundation elements in accordance with AISC 341 in structures assigned to SDC B, C, D, E or F.	Field test	Periodic

NOTES:

1. THE INSPECTION AND TESTING AGENT(S) SHALL BE ENGAGED BY THE OWNER OR THE OWNER'S AGENT, AND NOT BY THE CONTRACTOR OR SUBCONTRACTOR WHOSE WORK IS TO BE INSPECTED OR TESTED. ANY CONFLICT OF INTEREST MUST BE DISCLOSED TO THE BUILDING OFFICIAL PRIOR TO COMMENCING WORK. THE QUALIFICATIONS OF THE SPECIAL INSPECTOR(S) AND/OR TESTING AGENCIES MAY BE SUBJECT TO THE APPROVAL OF THE BUILDING OFFICIAL AND/OR THE DESIGN PROFESSIONAL.
2. THE LIST OF SPECIAL INSPECTORS MAY BE SUBMITTED AS A SEPARATE DOCUMENT, IF NOTED SO ABOVE.
3. SHOP INSPECTIONS OF FABRICATED ITEMS ARE NOT REQUIRED WHERE THE FABRICATOR IS APPROVED IN ACCORDANCE WITH IBC SECTION 1704.2.5.1 AND LISTED IN ACTIVITY 1709.2.
4. OBSERVE: OBSERVE ON A RANDOM BASIS, OPERATIONS NEED NOT BE DELAYED PENDING THESE INSPECTIONS. PERFORM: THESE TASKS SHALL BE PERFORMED FOR EACH WELDED JOINT, BOLTED CONNECTION, OR STEEL ELEMENT.
5. NDT OF WELDS COMPLETED IN AN APPROVED FABRICATOR'S SHOP MAY BE PERFORMED BY THAT FABRICATOR WHEN APPROVED BY THE AHJ. REFER TO AISC 360, N6.

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PROFESSIONAL ENGINEER

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DATE: 06/17/2026

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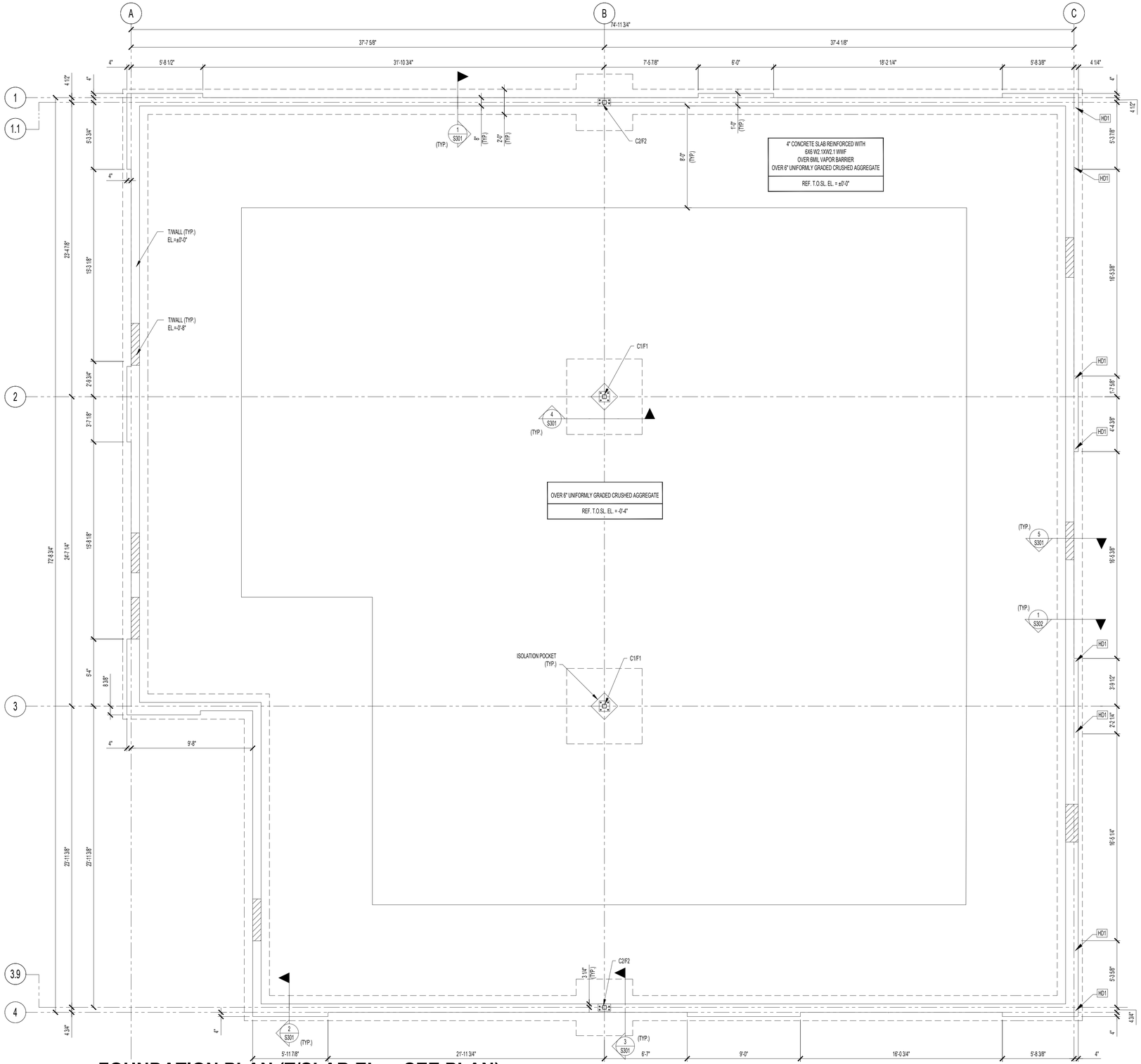
Project Number:	**KPJ-25-0282**

SHEET NAME:

SPECIAL INSPECTION NOTES

SHEET NUMBER:

S-104



1 FOUNDATION PLAN (T/SLAB EL. = SEE PLAN)

SCALE: 1/4" = 1'-0"

FOOTING SCHEDULE						
FOOTING MARK	SIZE			TOP OF FOOTING EL.	REINFORCEMENT	DETAIL
	WIDTH	LENGTH	THICKNESS			
F1	6'-0"	6'-0"	1'-0"	-1'-0"	(7) #5 BARS BOTTOM E.W.	4/S301
F2	4'-6"	4'-6"	1'-0"	-2'-6"	(6) #5 BARS BOTTOM E.W.	3/S301

LEGEND:

8" DEPRESSED FOUNDATION WALL (SEE NOTE 10)

HD1 — INDICATES HOLDOWN- SIMPSON HOLDOWN HDU4-SDS2.5 (TYP) WITH 5/8" Ø TYPE "A" ANCHOR BOLT WITH MIN. 18" EMBED. REFER DETAIL 6/S-301.

NOTES:

1. SEE S-101 & S-102 FOR GENERAL NOTES.
2. SEE S-103 & S-104 FOR SPECIAL INSPECTION NOTES.
3. SEE SHEET S-301 FOR TYPICAL FOUNDATION DETAILS.
4. FX INDICATES FOOTING MARK.
5. CX INDICATES STEEL COLUMN MARK. SEE SHEET S-401 FOR COLUMN SCHEDULE & BASE PLATE DETAILS.
6. T/SLAB ON GRADE ELEVATION: +0'-0" U.N.O.
7. VERIFY ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS.
8. THE SUBGRADE SHALL BE PROOFROLLED AND SOFT SOILS SHALL BE REMOVED AND REPLACED AS REQUIRED UNDER THE SUPERVISION OF A QUALIFIED GEOTECHINICAL ENGINEER.
9. CONTROL JOINTS ARE SHOWN IN SLAB ON GRADE TO MINIMIZE COSMETIC SHRINKAGE CRACKS. SPACING OF CONTROL JOINT IN SLAB IS 12'-0" (MAX.) O.C. (SEE GENERAL NOTES).
10. DEPRESSED WALL SHOWN ON THE DRAWING ARE APPROXIMATE LOCATION. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS.

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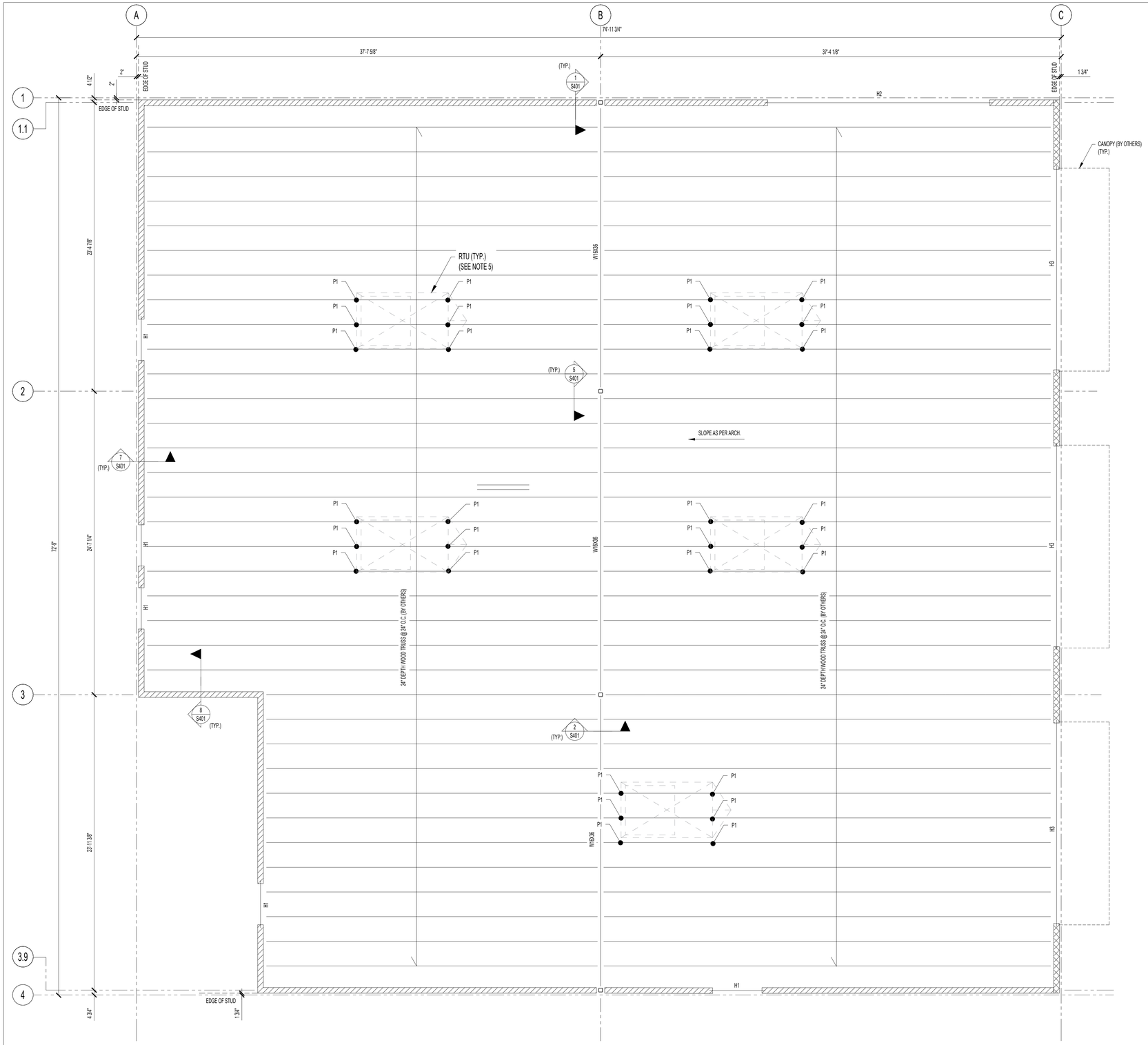
Project Number:	**KPJ-25-0282**

SHEET NAME:

FOUNDATION PLAN

SHEET NUMBER:

S-201

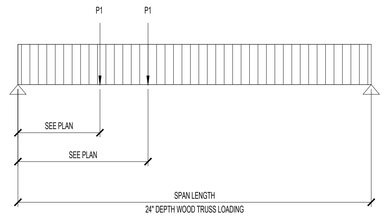


1 ROOF FRAMING PLAN (T.O.S. EL. = +12' - 3" (U.N.O.))

SCALE: 1/4" = 1'-0"

(TRUSS BOTTOM BEARING EL. = + 12' - 4 1/2" (U.N.O.))

D = 20 PSF
Lr = 20 PSF
S (BALANCED) = 20 PSF



NOTES:

1. DESIGN JOISTS WITH POINT LOAD CREATING MAXIMUM MOMENT AND SHEAR IN LOCATION EACH DIRECTION.
2. P1=600 # SHALL BE APPLIED TO EACH TRUSS FOR THE RTU SUPPORTED.
3. SEE SHEET S-203 FOR SNOW DRIFT LOAD DIAGRAM.

2 RTU LOADING DIAGRAM

SCALE: N.T.S.

BEAM REACTION SCHEDULE		
BEAM SIZE	ASD SHEAR	ASD MOMENT (FOR MOMENT CONNECTION ONLY)
W16X36	35 KIPS	-

		HEADER SCHEDULE		
MARK	SIZE	NUMBER OF JACK STUDS	NUMBER OF KING STUDS	HEADER TO KING NAILING
H1	(3) 2x6	2	2	(4) NAILS PER PLY
H2	(3)1.75"x11.25" LVL 2.0E	3	3	(6) NAILS PER PLY
H3	(3) 1.75"x18" LVL 2.0E	3	3	(10) NAILS PER PLY

LEGEND:

INDICATES 2x6 LOAD BEARING/SHEAR WALL, 16" O.C. STUD SPACING,WIZIP SYSTEM R-9 SHEATHING WITH 0.131", SHANK NAILS 3" O.C. EDGE SPACING 12" O.C. FIELD NAILING WITH MIN. 1 1/2" PENETRATION INTO THE FRAMING ON THE EXTERIOR FACE OF THE SHEAR WALL, BLOCKED, (2) 2X6 END POST

INDICATES 2x6 LOAD BEARING/SHEAR WALL, 7/16" OSB SHEATHING, 16" STUD SPACING, 8d NAIL 2" O.C. EDGE SPACING 12" O.C. FIELD SPACING, BLOCKED, (3) 2x6 END POST

NOTES:

1. SEE S-101 & S102 FOR GENERAL NOTES AND SPECIAL INSPECTION NOTES.
2. SEE S-401 & S402 FOR STEEL & WOOD DETAILS.
3. OPENINGS SHOWN ON THE DRAWING ARE APPROXIMATE LOCATION. SEE ARCHITECTURAL DRAWINGS FOR WALL LOCATIONS AND DOOR OPENINGS.
4. VERIFY ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS.
5. RTUS & OPENINGS SHOWN ON THE DRAWING ARE APPROXIMATE LOCATION. ANY ROOF OPENING AND LOCATION SHALL BE COORDINATED WITH MEP DRAWINGS AND CONTRACTORS.
6. SEE MECHANICAL DRAWINGS FOR RTU LOAD PRIOR TO FABRICATION OF TRUSSES. CONFIRM THE LOADS WITH ARCHITECT.

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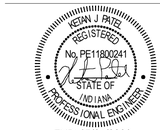

EXP: 07/31/2026
DATE: 06/17/2026

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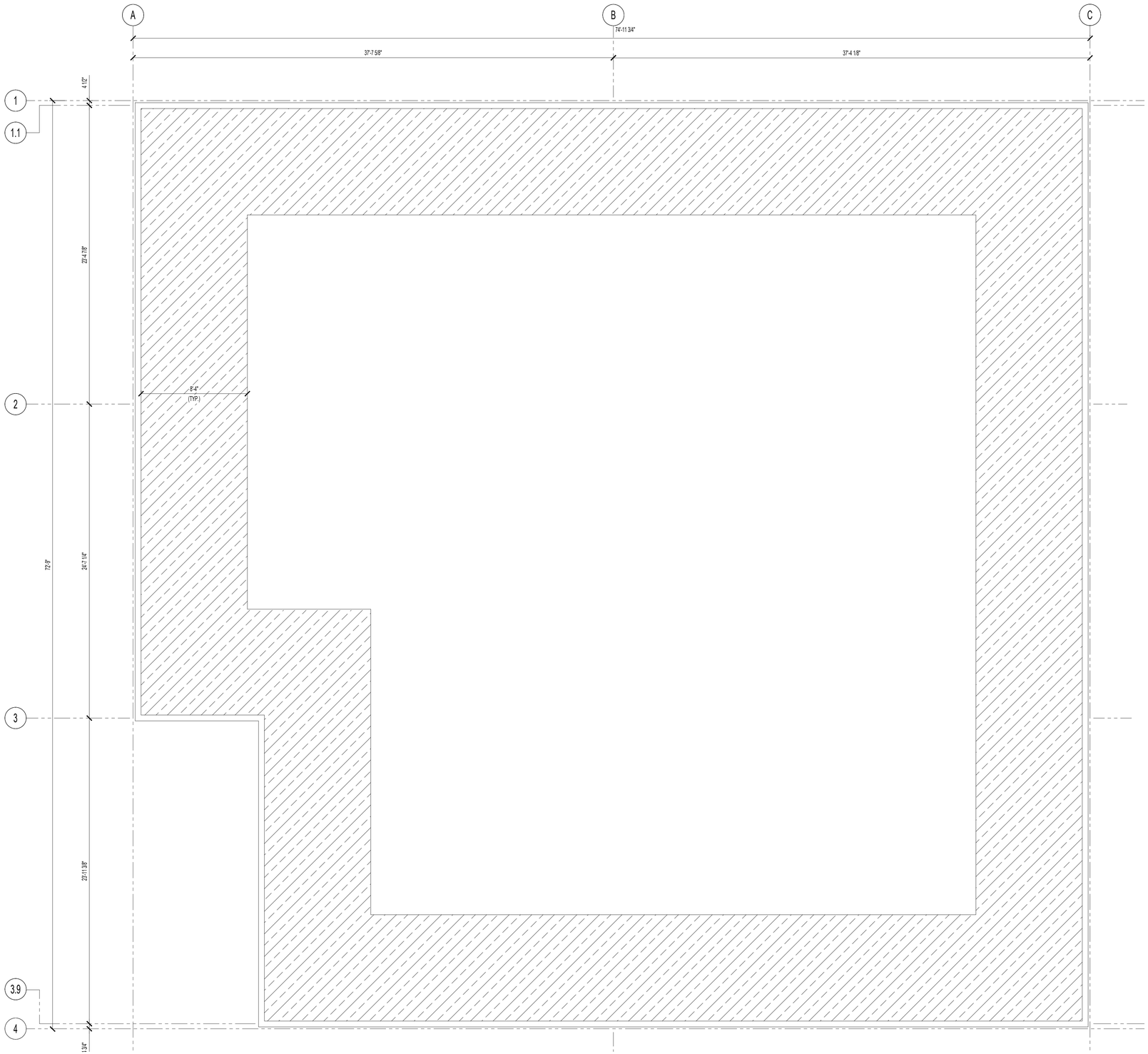
Project Number:	**KPJ-25-0282**

SHEET NAME:

ROOF FRAMING PLAN

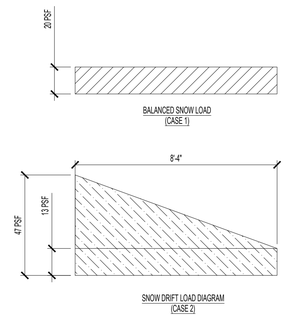
SHEET NUMBER:

S-202



1 SNOW DRIFT PLAN

SCALE: 1/4" = 1'-0"



2 SNOW LOADING DIAGRAM

SCALE: N.T.S.

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Approved by:	KP
Project Number:	**KPJ-25-0282**

SHEET NAME:

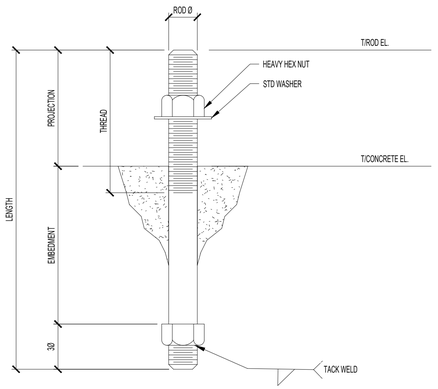
SNOW DRIFT PLAN

SHEET NUMBER:

S-203

NOTE:

ANCHOR BOLT SHALL MEET CONCRETE COVER REQUIREMENT AS CONCRETE REINFORCING STEEL, SEE S-101.

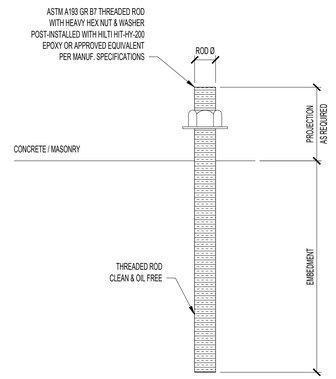


8 ANCHOR BOLT TYPE "A" (CAST-IN-PLACE)

SCALE: N.T.S.

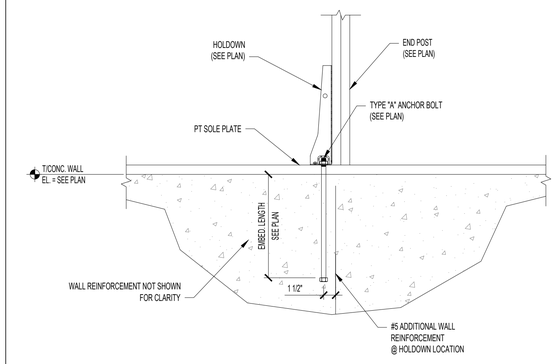
NOTE:

ANCHOR BOLT SHALL MEET CONCRETE COVER REQUIREMENT AS CONCRETE REINFORCING STEEL, SEE S-101.



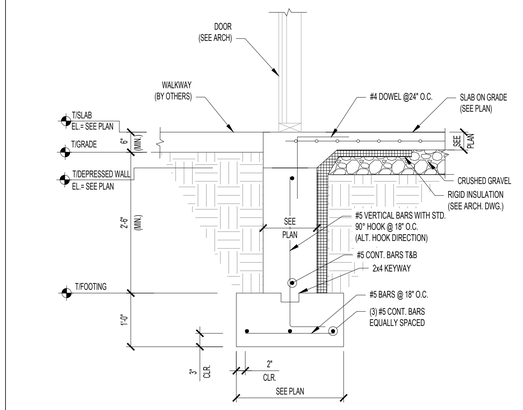
7 ANCHOR BOLT TYPE "B" (POST-INSTALLED)

SCALE: N.T.S.



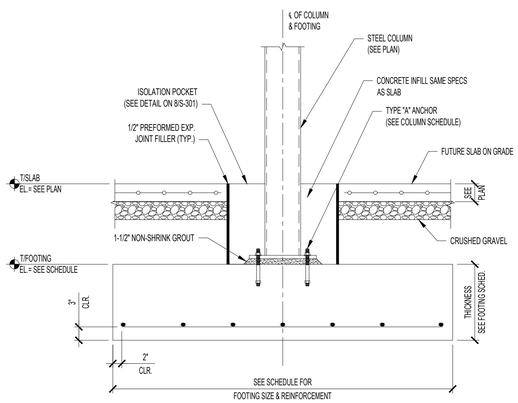
6 TYPICAL SUPPLEMENTAL REINFORCEMENT FOR HOLDOWNS

SCALE: 1" = 1'-0"



5 TYPICAL FOUNDATION DETAIL AT DOOR

SCALE: 3/4" = 1'-0"

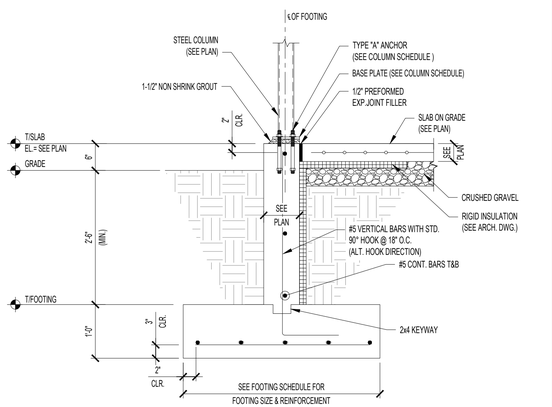


4 TYPICAL INTERIOR FOOTING DETAIL

SCALE: N.T.S.

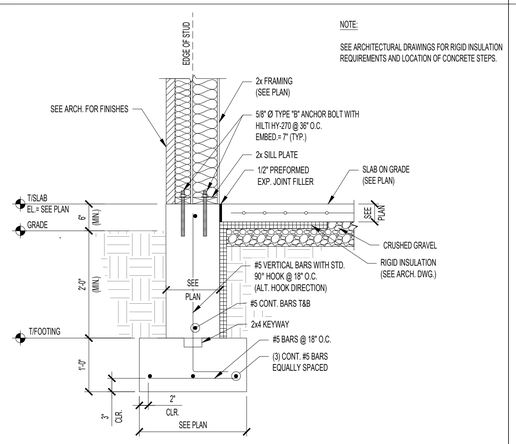
NOTE:

SEE ARCHITECTURAL DRAWINGS FOR RIGID INSULATION REQUIREMENTS AND LOCATION OF CONCRETE STEPS.



3 TYPICAL EXTERIOR FOUNDATION DETAIL FOR STEEL POST

SCALE: N.T.S.

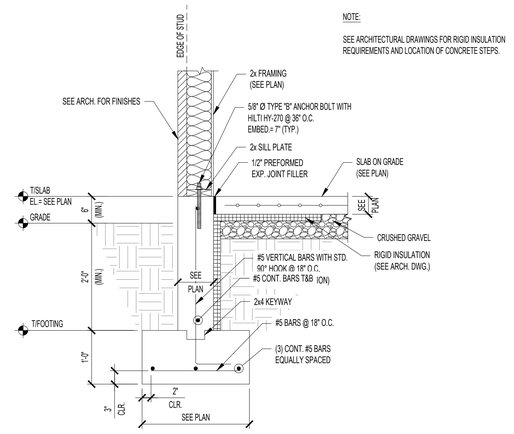


2 TYPICAL EXTERIOR WALL FOUNDATION DETAIL

SCALE: 3/4" = 1'-0"

NOTE:

SEE ARCHITECTURAL DRAWINGS FOR RIGID INSULATION REQUIREMENTS AND LOCATION OF CONCRETE STEPS.



1 TYPICAL EXTERIOR WALL FOUNDATION DETAIL

SCALE: 3/4" = 1'-0"

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SAGAMORE PARKWAY/CUMBERLAND AVENUE, LOT 13
WEST LAFAYETTE, IN 47906

REV	REV DESCRIPTION	DATE
	ISSUED FOR PERMIT	04/27/26

KETAN J PATEL
REGISTERED
No. PE11800241
STATE OF
INDIANA
PROFESSIONAL ENGINEER

EXP: 07/31/2026
DATE: 06/17/2026

DESIGN PROFESSIONAL SEAL

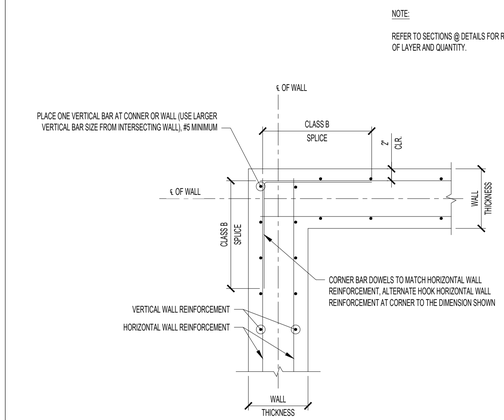
Drawn / Designed by:	NA/KV
Reviewed by:	RU
Approved by:	KP
Project Number:	**KPJ-25-0282**

SHEET NAME:

FOUNDATION PLAN

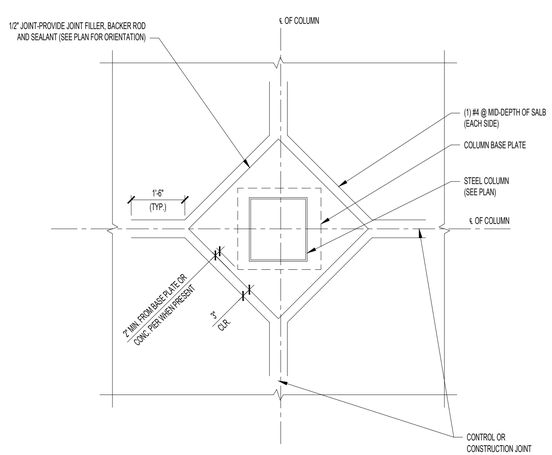
SHEET NUMBER:

S-301



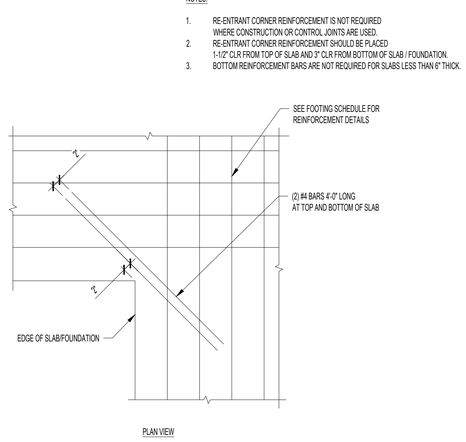
4 REINFORCEMENT SPLICE DETAIL AT CORNER WALL

SCALE: N.T.S.



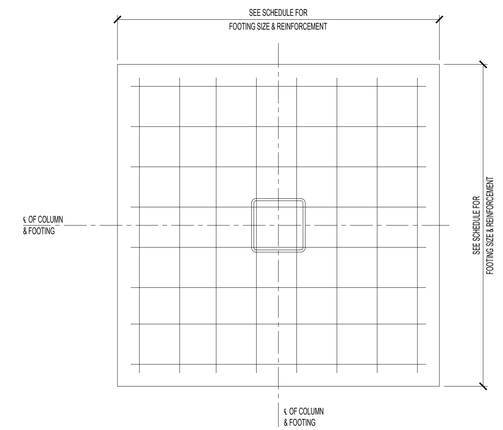
3 TYPICAL COLUMN ISOLATION POCKET

SCALE: N.T.S.



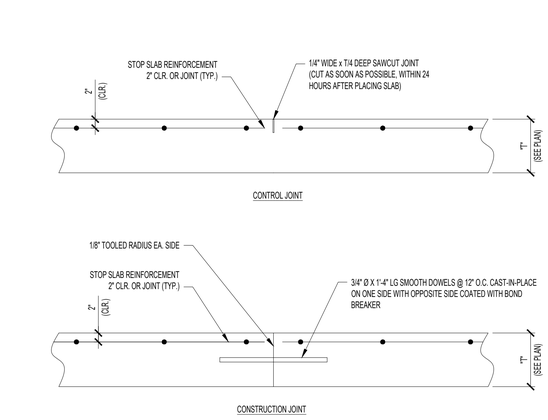
2 TYPICAL SLAB RE-ENTRANT CORNER REINFORCEMENT

SCALE: N.T.S.



5 FOUNDATION REINFORCEMENT ARRANGEMENT

SCALE: N.T.S.



1 TYPICAL SLAB ON GRADE DETAIL

SCALE: N.T.S.

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KEVIN J. PATEL REGISTERED No. PE11800241 STATE OF INDIANA PROFESSIONAL ENGINEER

EXP: 07/31/2026

DATE: 06/17/2026

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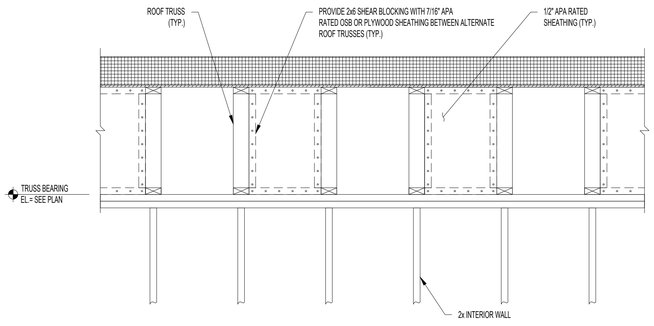
Project Number:	**KPJ-25-0282**

SHEET NAME:

FOUNDATION PLAN

SHEET NUMBER:

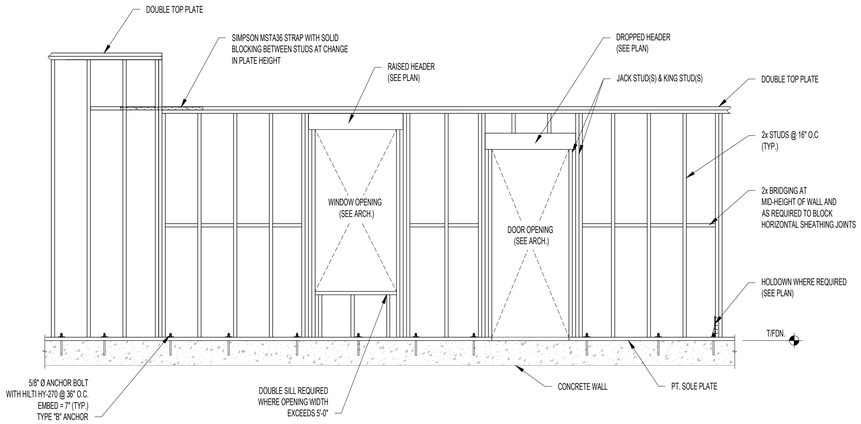
S-302



TYPICAL ROOF TRUSS SHEAR BLOCKING AT
INTERIOR LOAD-BEARING/SHEAR WALL

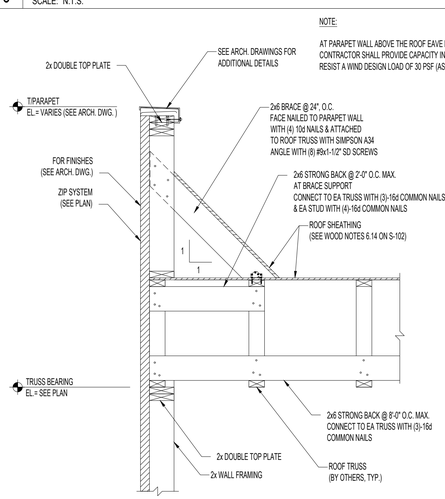
10 ROOF TRUSS DETAIL

SCALE: N.T.S.



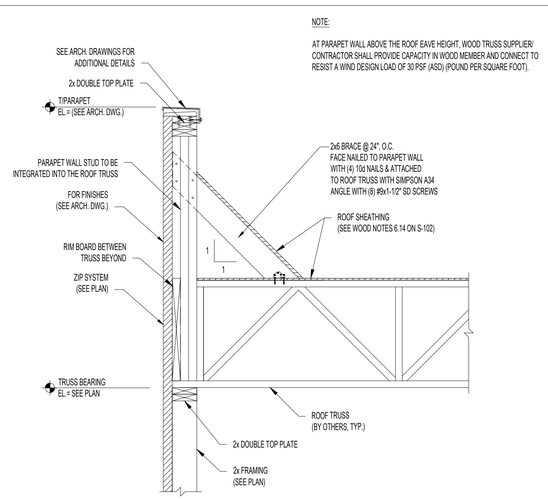
9 TYPICAL WALL FRAMING DETAIL

SCALE: N.T.S.



8 TYP. PARAPET AT PARALLEL ROOF TRUSS AT EXTERIOR WALL

SCALE: 3/4" = 1'-0"



7 TYP. PARAPET AT PERP. ROOF TRUSS AT EXTERIOR WALL

SCALE: 3/4" = 1'-0"

NOTES:

1. NUMBER OF BOLTS SHOWN ARE MINIMUM.
2. CONNECTION SHALL BE DESIGNED PER GENERAL NOTES.

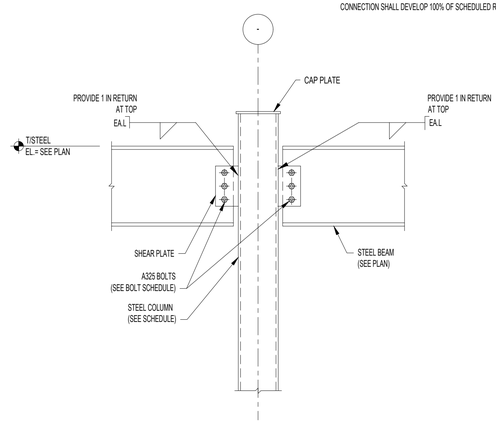
BEAM DEPTH	ANGLE/PLATE THICKNESS	MIN NO. 3/4" Ø A325 BOLTS
8"	1/4"	2
10"	5/16"	2
12"	5/16"	3
14"	5/16"	3
16"	5/16"	4
18"	5/16"	5
21"	5/16"	6
24"	3/8"	6
27"	3/8"	7
30"	3/8"	8

6 BOLT SCHEDULE

SCALE: N.T.S.

NOTE:

CONNECTION SHALL DEVELOP 100% OF SCHEDULED REACTION.



5 TYPICAL BEAM TOP COLUMN SHEAR CONNECTION

SCALE: N.T.S.

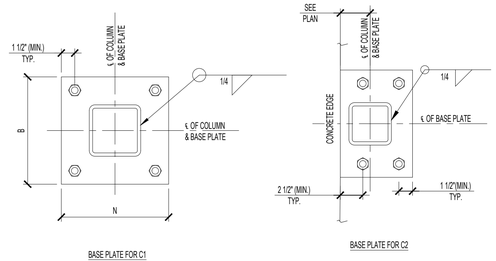
COLUMN SCHEDULE (min Fy=46ksi)		
COLUMN MARK / LEVEL	C1	C2
ROOF LEVEL EL=SEE PLAN	HSS 4X4X3/8	HSS 4X4X3/8
FOUNDATION LEVEL EL =SEE PLAN		
BASE PLATE t x N x B	3/4"X10"X10"	3/4"X12"X7"
ANCHOR BOLTS	(4) 3/4" Ø TYPE "A" ASTM F1554 GR.36	(4) 1/2" Ø TYPE "A" ASTM F1554 GR.36
REMARKS	ANCHOR EMBED. = 12" (MIN.)	ANCHOR EMBED. = 12" (MIN.)

4 COLUMN SCHEDULE

SCALE: N.T.S.

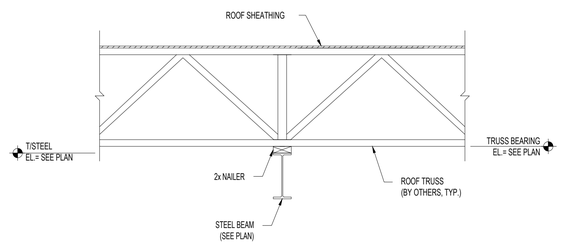
NOTE:

SEE COLUMN SCHEDULE FOR BASE PLATE DIMENSIONS.



3 TYPICAL BASE PLATE DETAILS

SCALE: N.T.S.

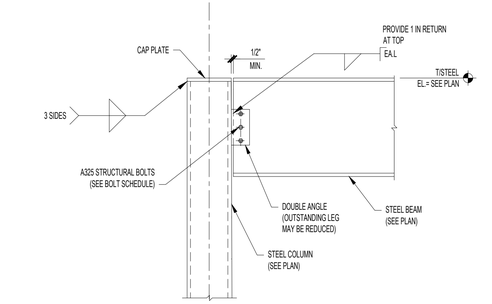


2 TYPICAL STEEL BEAM SUPPORT WOOD TRUSS

SCALE: 3/4" = 1'-0"

NOTE:

CONNECTION SHALL DEVELOP 100% OF SCHEDULED REACTION.



1 TYPICAL BEAM TOP COLUMN SHEAR CONNECTION

SCALE: 3/4" = 1'-0"

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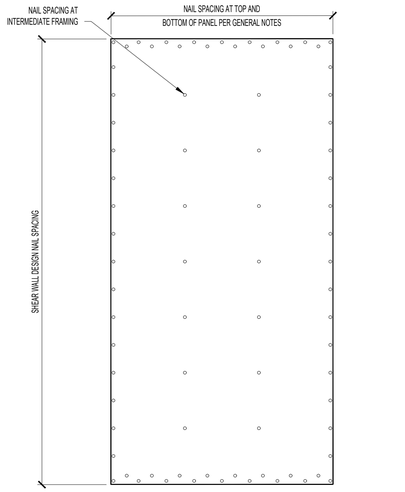
Approved by:	KP
Project Number:	**KPJ-25-0282**

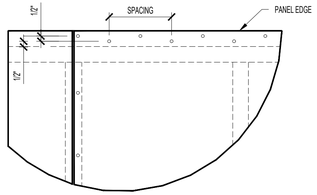
SHEET NAME:

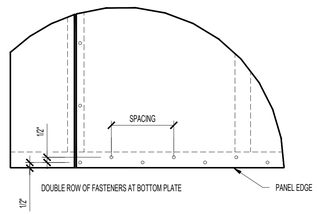
STEEL AND WOOD DETAILS

SHEET NUMBER:

S-401



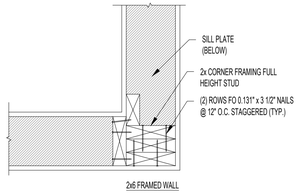


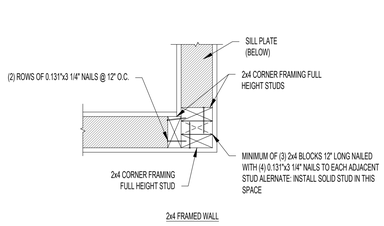


7 TYPICAL SHEAR WALL NAILING PATTERN DETAIL
SCALE: N.T.S.

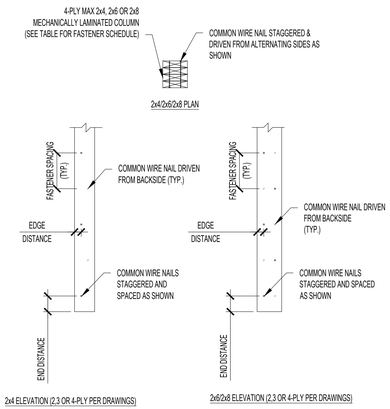
BUILT-UP LUMBER COLUMN FASTENER TABLE					
COLUMN	FASTENER SIZE	FASTENER SPACING	NUMBER OF ROWS	EDGE DISTANCE	EDGE DISTANCE
2-PLY, 2x4	10d (0.148"x3")	6"	1	1"	2-1/2"
2-PLY, 2x6	10d (0.148"x3")	8"	2	1-1/2"	
2-PLY, 2x8	10d (0.148"x3")	8"	2	2-1/8"	
3-PLY, 2x4	30d (0.207"x4 1/2")	8"	1	1-1/2"	3-1/2"
3-PLY, 2x6	30d (0.207"x4 1/2")	8"	2	1-1/2"	
3-PLY, 2x8	30d (0.207"x4 1/2")	8"	2	2-1/8"	
4-PLY, 2x4	60d (0.263"x6")	8"	1	1-1/2"	4"
4-PLY, 2x6	60d (0.263"x6")	8"	2	1-1/2"	
4-PLY, 2x8	60d (0.263"x6")	8"	2	2-1/8"	

10 TYPICAL BUILT-UP COLUMN LUMBER FASTENER SCHEDULE
SCALE: N.T.S.

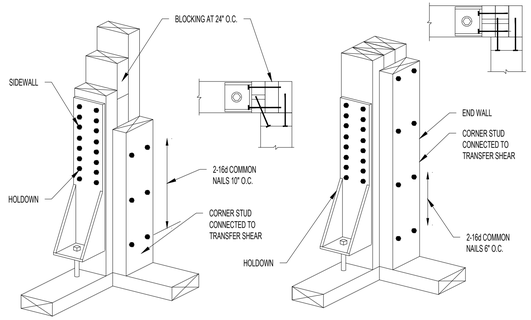




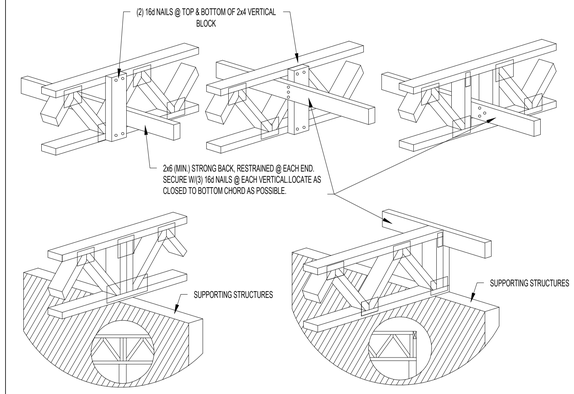
6 TYPICAL WOOD WALL FRAMING DETAIL
SCALE: N.T.S.



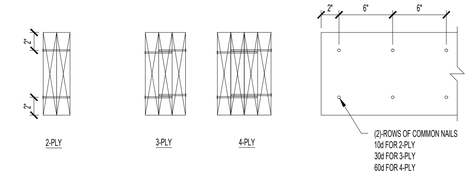
9 TYPICAL BUILT-UP LUMBER COLUMN DETAIL
SCALE: N.T.S.



5 TYPICAL HOLDOWN
SCALE: N.T.S.



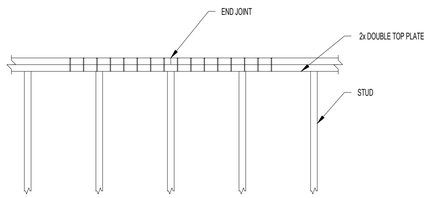
8 TYPICAL TRUSS DETAIL
SCALE: N.T.S.



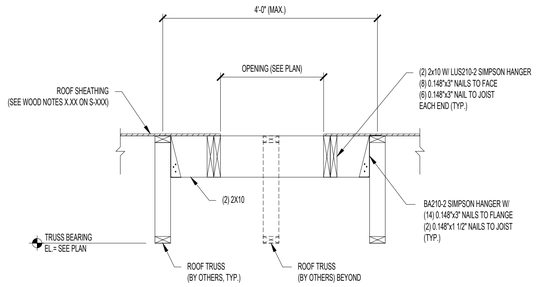
4 TYPICAL MULTI-PLY DETAIL
SCALE: N.T.S.

NOTE:

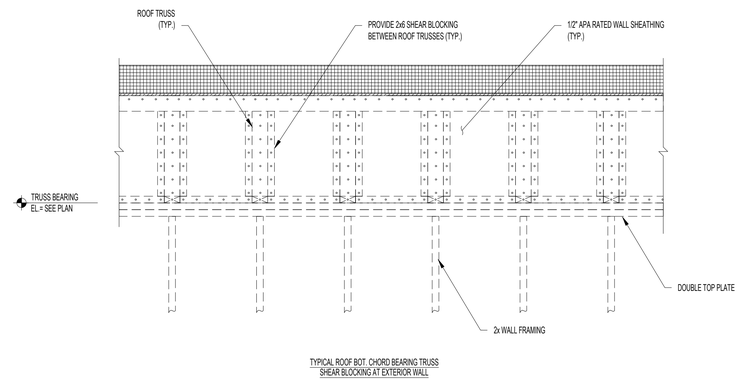
END JOINTS IN TOP PLATES SHOULD BE OFFSET A MINIMUM OF 48" AND OCCUR OVER STUDS.



3 TYPICAL TOP PLATE SPLICE DETAIL
SCALE: N.T.S.



2 TYPICAL ROOF OPENING DETAIL
SCALE: N.T.S.



TYPICAL ROOF BOT. CHORD BEARING TRUSS SHEAR BLOCKING AT EXTERIOR WALL

1 ROOF TRUSS DETAIL
SCALE: N.T.S.

NOTE:

FOR TOP LOADED BUILT-UP BEAMS WHERE THE VERTICAL LOAD IS EQUALLY DISTRIBUTED AMONG ALL PLYS, FACE NAILS MAY BE SPACED @ 12 O.C.

OFFICIAL SEAL

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Approved by:	KP
Project Number:	**KPJ-25-0282**

SHEET NAME:

WOOD DETAILS

SHEET NUMBER:

S-402